



**MELROSE PARK NORTH
PLANNING PROPOSAL**

CONCEPT STORMWATER STRATEGY

NOVEMBER 2020

Job No: FG486 File: Cover Page.doc	Date: November 2020 Rev No: 1.0	Principal: SAB Author: SAB
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M Projects
Suite C2.08, Level 2
22-36 Mountain Street NSW 2007

Job No. FG486

Attn: Mr Miled Akle

5 November 2020

Re: Melrose Park Flooding and Drainage Investigation – VRS and PP Development Sites

Dear Sir

This letter report deals with the initial findings of a flooding and drainage investigation that was undertaken into the impact that the redevelopment of Melrose Park North as a whole would have on flood behaviour.

1. Background

Payce and Sekisui House Australia (**the developers**) are in the process of redeveloping Melrose Park North which is bounded by Victoria Road to the north, Wharf Road to the east, existing residential development to the west and Hope Street to the south. Melrose Park North has been divided into two parcels of land for staging purposes, these being the Victoria Road (**VRS**) and Planning Proposal (**PP**) sites.

While earlier flooding and drainage investigations have been undertaken by others in recent years, Lyall & Associates was recently engaged by the developers to undertake further, more detailed flood modelling for both the VRS and PP sites. Our letter of 29 July 2020 provided background to the development of the hydrologic (DRAINS) and hydraulic (TUFLOW) models (collectively referred to herein as "**the flood models**") that were used to define the nature of flooding in the vicinity of the VRS and PP sites under pre-developed conditions, as well as those under post-VRS developed conditions. A subsequent letter dated 24 August 2020 provided additional information relating to post-VRS development conditions that were requested by City of Parramatta Council (**CoPC**).¹

The following sections of this letter set out the modifications that have been made to the flood models in order that they reflected post-VRS and PP development conditions, as well as the impact that the future development within Melrose Park North as a whole would have on flood behaviour for design storms with Annual Exceedance Probabilities (**AEPs**) of 5% (1 in 20) and 1% (1 in 100).

2. Overview of Post-VRS and PP Development Conditions

Figure 1 (7 sheets) shows the key features of the upgraded road and stormwater drainage system under post-VRS and PP development conditions:

- The widening of Victoria Road to the south, as well as the upgrade of Kissing Point Road and Wharf Road intersections.
- The upgrade of the existing stormwater drainage system in Victoria Road to improve its hydrologic standard to a minimum of 5% (1 in 20) AEP where practical.

¹ It is noted that further refinements have been made to the structure of the flood models which are representative of post-VRS development conditions and it is these models that have been used as the basis of the present investigation.

- An internal road network which has connections to Victoria Road, Wharf Road and Hope Street.
- Multi-storey buildings within the super-lots that are formed by the internal road network. Development within each super-lot will incorporate on-site detention facilities which will be sized in accordance with CoPC's requirements.
- An internal piped stormwater drainage system which has been sized to convey flows generated by storms that are up to 5% AEP in intensity.
- A detention basin which would be constructed adjacent to the natural low point in Wharf Road (**Wharf Road Detention Basin**). The purpose of the Wharf Road Detention Basin is to ensure that flood behaviour is not exacerbated in existing residential development that is located on the eastern side of Wharf Road in the City of Ryde Local Government Area (**LGA**).
- A detention basin in the south-west corner of Melrose Park North (**Hope Street Detention Basin**). The purpose of the Hope Street Detention Basin is to ensure that flood behaviour is not exacerbated in existing and future development that is located on the southern side of Hope Street in the CoPC LGA.

The following section sets out the changes that were made to the structure of the flood models to reflect these key features.

3. Background to the Update of the Flood Models

The following changes were made to the post-VRS development flood models in order that they reflected post-VRS and PP development conditions:

- A 3D design model which has been developed by CCHD for the widening of Victoria Road was incorporated in the TUFLOW model.
- Modifications were made to the configuration of the existing stormwater drainage system in Victoria Road which are aimed at improving the minimum hydrologic standard of the road to 5% AEP where practical while not exacerbating flooding conditions in adjacent development. Both the DRAINS and TUFLOW models were updated to reflect the upgraded stormwater drainage system in Victoria Road.
- A 3D design model which has been developed by Northrop of the internal road network was incorporated in the TUFLOW model.
- An internal pit and pipe network was developed for the PP site which had a minimum hydrologic standard of 5% AEP. As per CoPC's requirements, blockage factors of 20% and 50% were applied to on-grade and sag pits, respectively. Both the DRAINS and TUFLOW models were updated to reflect the new stormwater drainage system internal to the PP site.²
- The four temporary OSD tanks which are located along the southern side of East West Road 2 (**EWR2**) were removed from the model and the pipe drainage system beneath the southern kerbline of the road made continuous along its full length.
- The 1200 mm diameter pipe which crosses North South Road 4 (**NSR4**) was removed from the model and the upstream system connected into the new internal piped drainage system associated with the PP site.

² Note that the DRAINS model only incorporated sub-catchments which were representative of uncontrolled flow off the internal road network, as well as other areas which would not be controlled by a dedicated on-site detention facility (e.g. in public space areas).

- Steady-state peak flows were applied to the TUFLOW model to represent the flow which would discharge to the new stormwater drainage system from each super-lot. The peak flow from each super-lot was derived using CoPC's "On-Site Detention Calculation Sheet – Version 9", completed copies of which are contained in **Annexure A** of this letter. Note that similar to the approach that was adopted for the VRS site, the on-site detention facilities have been sized to take into account the uncontrolled flow which would be associated with runoff from the adjacent internal road network. Note also that these calculations are preliminary in nature and would need to be reviewed following further design development.
- A 3D design of both the Wharf Road and Hope Street Detention Basins that were developed by Lyall & Associates using the 12d software were incorporated in the TUFLOW model. Note that the basins were sized to ensure that the depth of ponding does not exceed a maximum of 0.6 m in a 1% AEP storm event.

Figure 1 shows the layout of the updated TUFLOW model representing post-VRS and PP development conditions, while the following section of this letter sets out the key findings of the flood modelling that was undertaken as part of the present investigation.

4. Key Findings of Flooding Investigation

Figures 2 and 3 (3 sheets each) show the indicative extent and depth of inundation in the vicinity of Melrose Park North under post-VRS and PP development conditions for the 5% and 1% AEP storm events, while **Figures 4 and 5** (3 sheets each) show the impact that the implementation of the proposed drainage strategy as part of the future development would have on flood behaviour.

The key findings of the investigation were as follows:

- While it is feasible to improve the flood immunity of Victoria Road to a minimum of 5% AEP along its southern kerblines west of Wharf Road and along its northern kerblines west of Fitzgerald Road, major drainage improvements would be required extending south into Ryde Parramatta Golf Club in order to improve its hydrologic standard to the east of these two locations.
- The proposed drainage strategy would prevent surcharge of the internal drainage system for all storms up to 5% AEP in intensity and also improve the flood immunity of Wharf Road along most of its length. The exception is a short section of NSR2 immediately north of Hope Street where minor surcharge of the internal stormwater drainage system downstream of the Hope Street Detention Basin is shown to occur. Refer **Section 5** of this letter that sets out further investigations that will be undertaken to prevent this surcharge from occurring for all storms up to 5% AEP in intensity.
- Only shallow inundation would be experienced in the internal road network during a 1% AEP storm event, with depths of ponding in the Wharf Road and Hope Street Detention Basins limited to a maximum of 0.6 m during a storm of this intensity.
- The provision of on-site detention within each super-lot in combination with the Wharf Road and Hope Street Detention Basins will ensure that flooding conditions are not exacerbated to the east and south of Melrose Park North. In fact, implementation of the assessed drainage strategy will significantly reduce the degree to which flooding impacts existing residential development that is located to the east of Wharf Road in the City of Ryde LGA during storms up to 1% AEP in intensity.

As requested by CoPC, an assessment was also undertaken whereby it was assumed that both the existing and proposed stormwater drainage systems experience a complete blockage during a 1% AEP storm event. Note that it has been assumed that the on-site detention facilities within each super-lot which principally control runoff from roofed areas would continue to function as designed,

with the outflow discharging onto the surface of the internal road network rather than directly into the stormwater drainage system due to it being 100% blocked. It is understood that these conditions will form the basis of setting the minimum floor level requirements within each super-lot.

Figure 6 (7 sheets) shows the indicative extent and depth of inundation in the vicinity of Melrose Park North under post-VRS and PP development and complete blockage conditions for the 1% AEP storm event. Included on **Figure 6** are maximum 1% AEP water surface elevation contours at 0.2 m intervals, as well as peak overland flow rates at key locations.

5. Further Investigations

In addition to the above assessment, further modelling is presently being undertaken to:

- i. refine the proposed stormwater drainage strategy in the vicinity of the Hope Street Detention Basin so as to prevent surcharge of the internal stormwater drainage system for all storms up to 5% AEP in intensity;
- ii. define the nature of flooding under post-VRS and PP development conditions for the Probable Maximum Flood, and
- iii. assess the potential for future climate change to impact flood behaviour in and immediately adjacent to Melrose Park North.

We trust that the findings of the present investigation will assist the developers in completing the detailed design documentation for Melrose Park North. However, please do not hesitate to contact the undersigned should you have any queries or wish to discuss any aspect of our letter report.

Yours faithfully

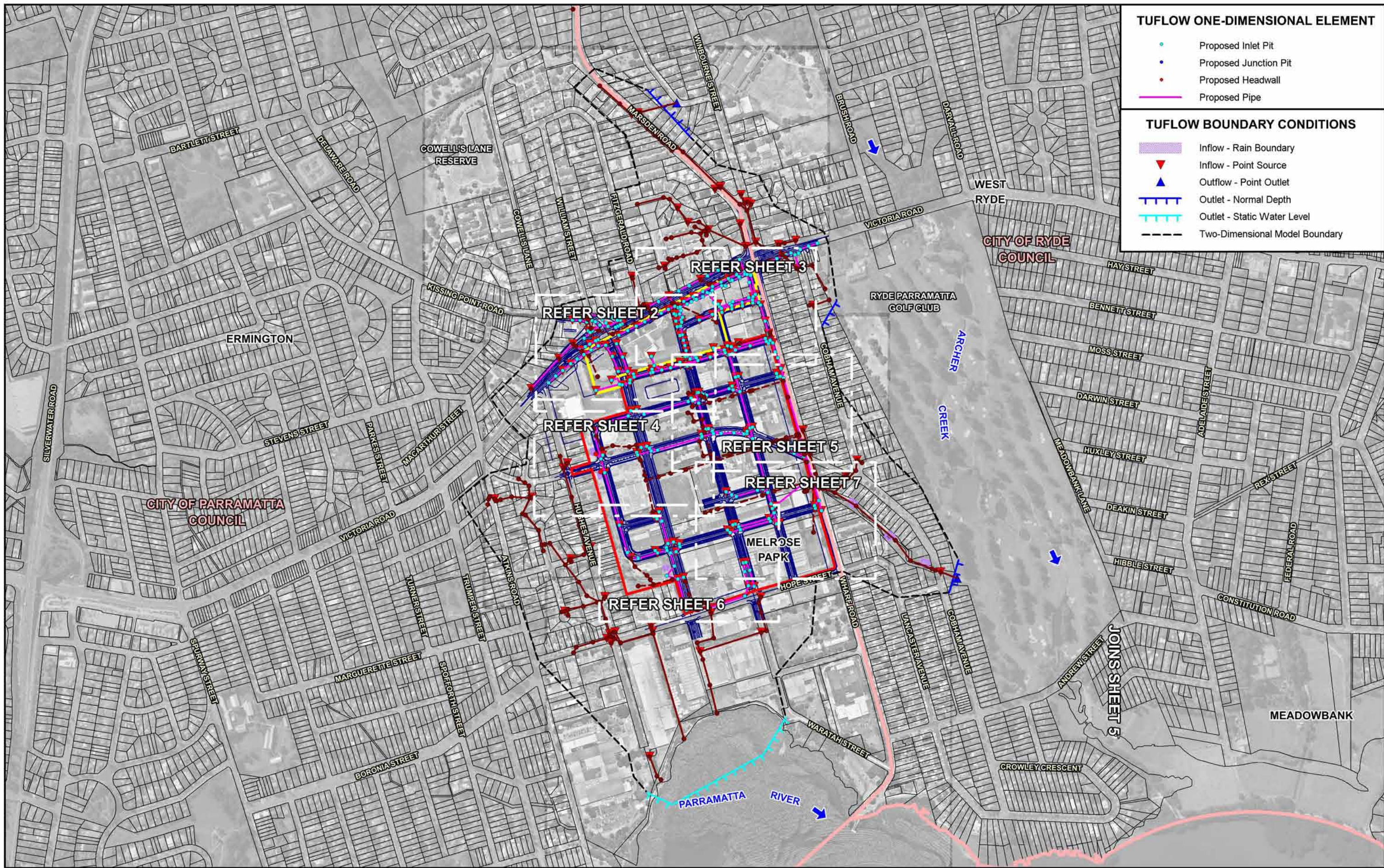
Lyll & Associates Consulting Water Engineers



Scott Button
Principal

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- Figure 2 Indicative Depth and Extent of Inundation - Post-VRS and PP Development Conditions - 5% AEP (3 sheets)
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- Figure 4 Impact of Proposed VRS and PP Development on Flood Behaviour - 5% AEP (3 sheets)
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- Figure 6 Indicative Extent and Depth of Inundation - Post-VRS and PP Development and Complete Blockage Conditions – 1% AEP (9 sheets)

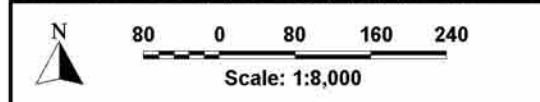


TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Inlet Pit
- Proposed Junction Pit
- Proposed Headwall
- Proposed Pipe

TUFLOW BOUNDARY CONDITIONS

- Inflow - Rain Boundary
- Inflow - Point Source
- Outflow - Point Outlet
- Outlet - Normal Depth
- Outlet - Static Water Level
- Two-Dimensional Model Boundary



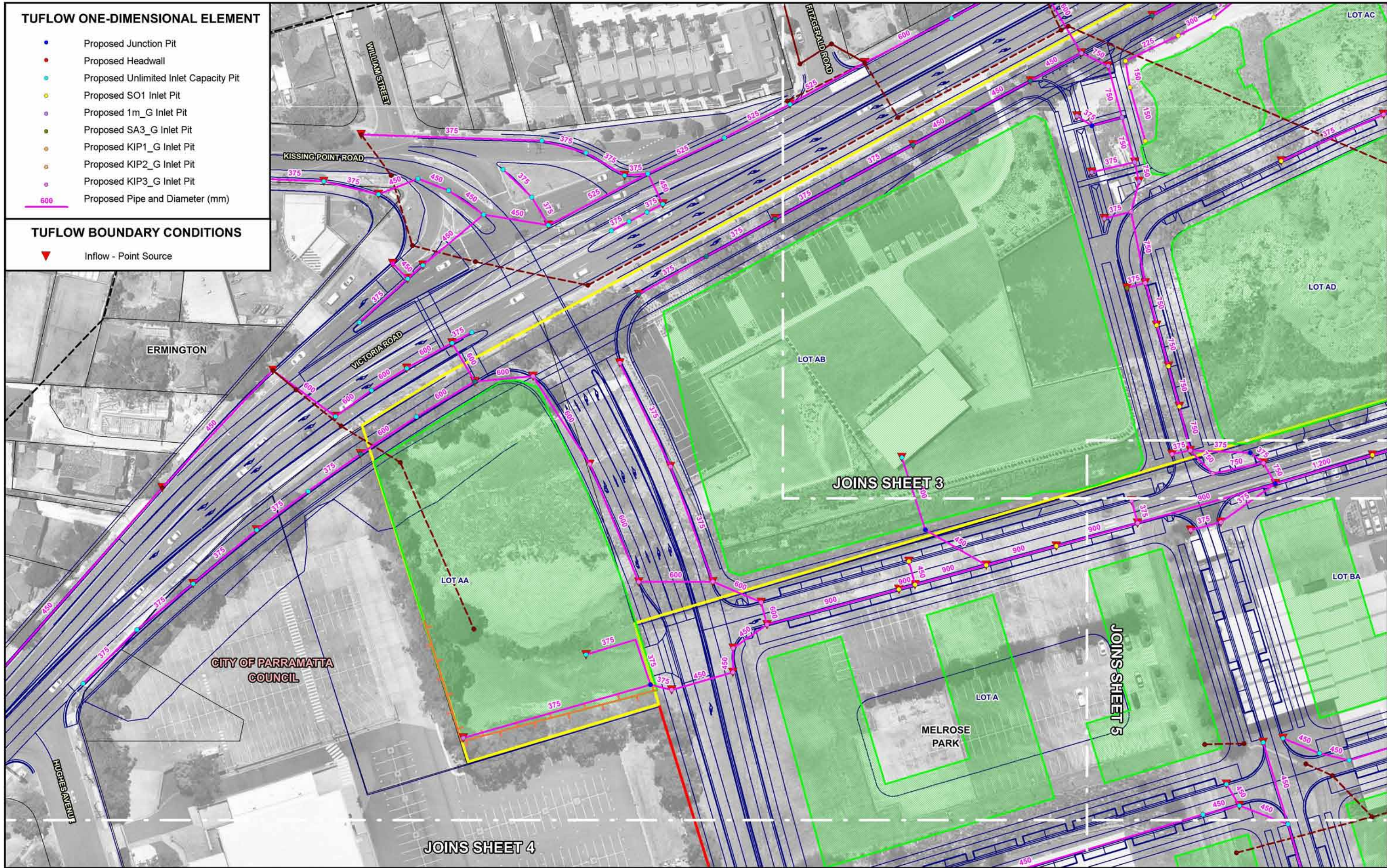
LEGEND

- LGA Boundary
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)
- Existing Drainage Network to Remain
- Existing Drainage Network to be Removed

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 1
(Sheet 1 of 7)

TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS



N
 10 0 10 20 30 m
 Scale: 1:1,000
Lyll & Associates

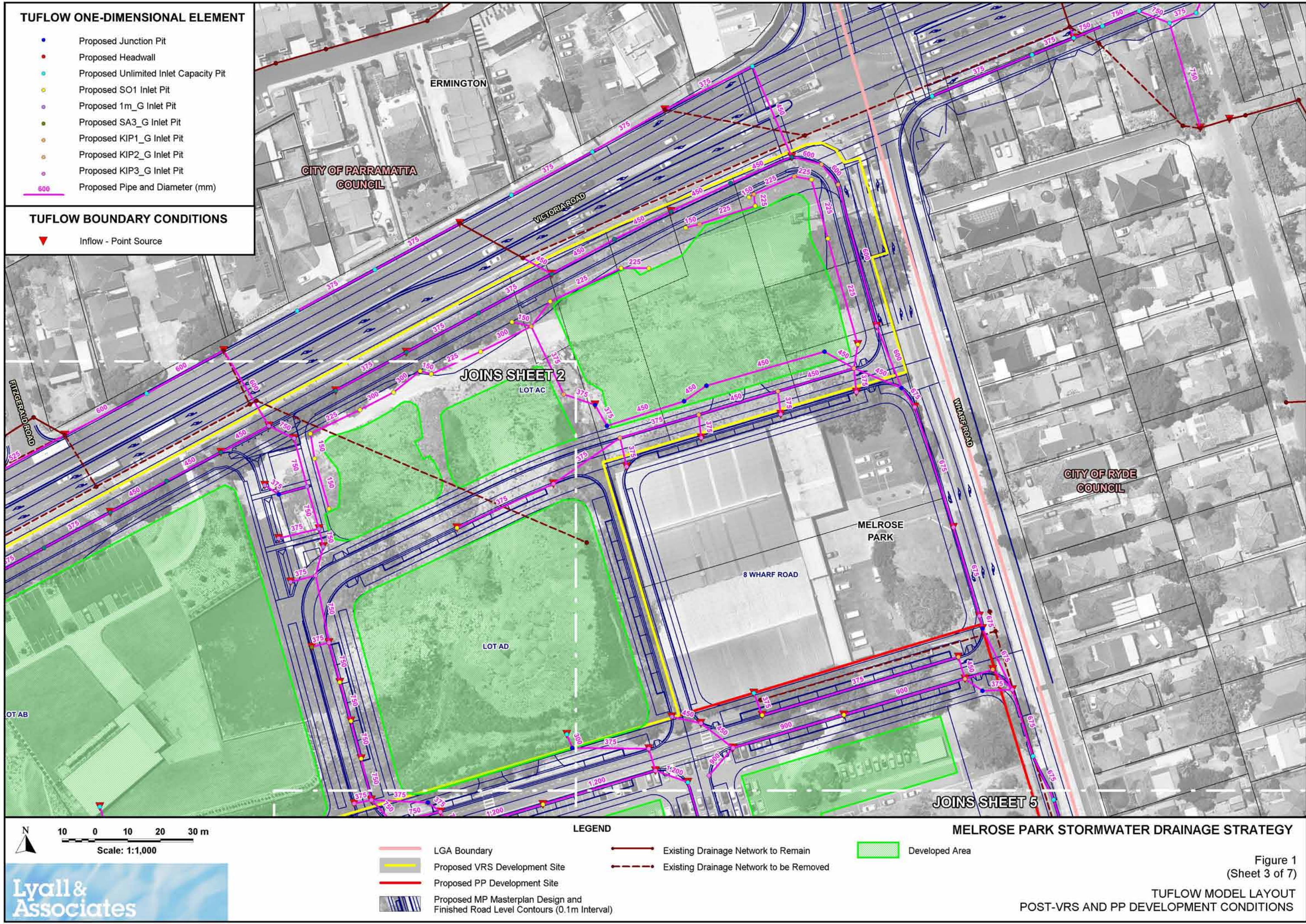
LEGEND

LGA Boundary	Existing Drainage Network to Remain	Developed Area
Proposed VRS Development Site	Existing Drainage Network to be Removed	
Proposed PP Development Site	Low Wall	
Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)		

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 1
(Sheet 2 of 7)

TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS

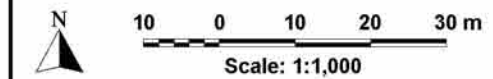


TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Junction Pit
- Proposed Headwall
- Proposed Unlimited Inlet Capacity Pit
- Proposed SO1 Inlet Pit
- Proposed 1m_G Inlet Pit
- Proposed SA3_G Inlet Pit
- Proposed KIP1_G Inlet Pit
- Proposed KIP2_G Inlet Pit
- Proposed KIP3_G Inlet Pit
- 600 Proposed Pipe and Diameter (mm)

TUFLOW BOUNDARY CONDITIONS

- ▼ Inflow - Point Source

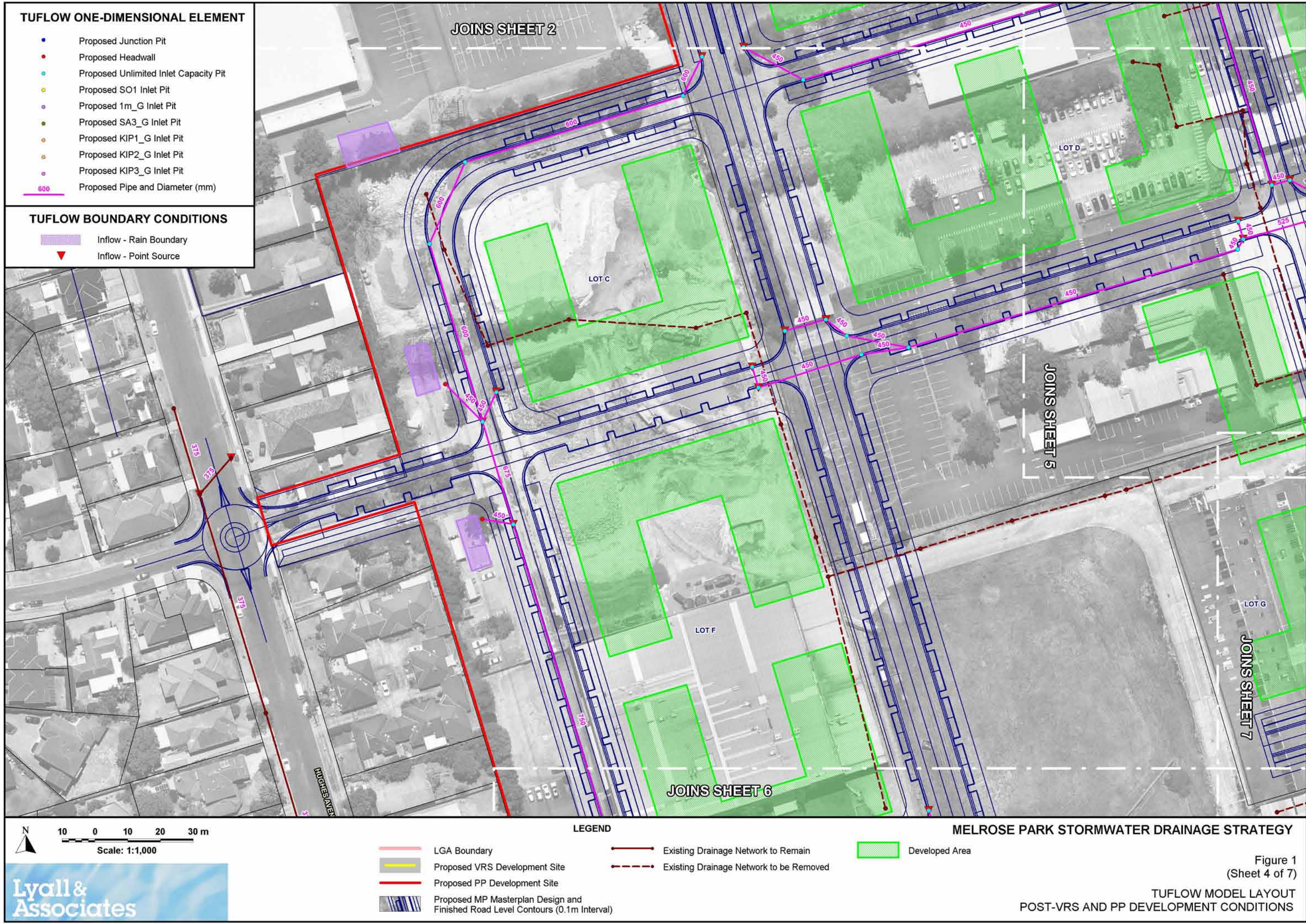


LEGEND

- LGA Boundary
- Existing Drainage Network to Remain
- Existing Drainage Network to be Removed
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)
- ▭ Developed Area

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 1
(Sheet 3 of 7)
TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS

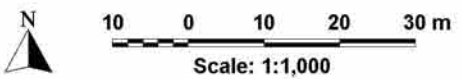


TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Junction Pit
- Proposed Headwall
- Proposed Unlimited Inlet Capacity Pit
- Proposed SO1 Inlet Pit
- Proposed 1m_G Inlet Pit
- Proposed SA3_G Inlet Pit
- Proposed KIP1_G Inlet Pit
- Proposed KIP2_G Inlet Pit
- Proposed KIP3_G Inlet Pit
- 600 Proposed Pipe and Diameter (mm)

TUFLOW BOUNDARY CONDITIONS

- Inflow - Rain Boundary
- ▼ Inflow - Point Source

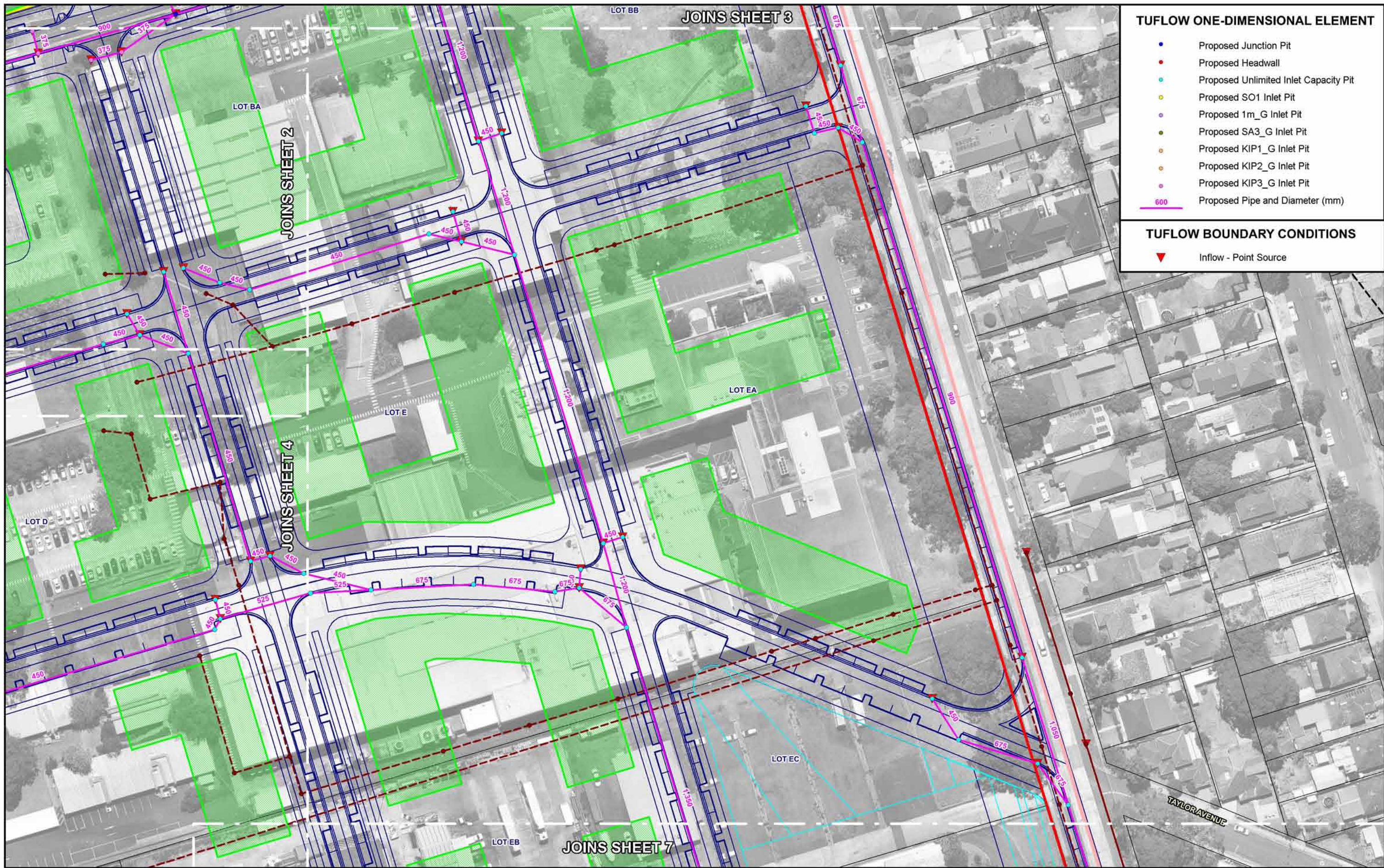


LEGEND

- LGA Boundary
- Existing Drainage Network to Remain
- Existing Drainage Network to be Removed
- Developed Area
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 1
(Sheet 4 of 7)
TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS

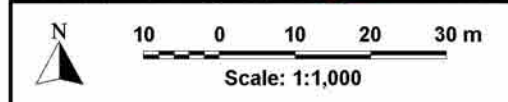


TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Junction Pit
- Proposed Headwall
- Proposed Unlimited Inlet Capacity Pit
- Proposed SO1 Inlet Pit
- Proposed 1m_G Inlet Pit
- Proposed SA3_G Inlet Pit
- Proposed KIP1_G Inlet Pit
- Proposed KIP2_G Inlet Pit
- Proposed KIP3_G Inlet Pit
- 600 Proposed Pipe and Diameter (mm)

TUFLOW BOUNDARY CONDITIONS

- ▼ Inflow - Point Source

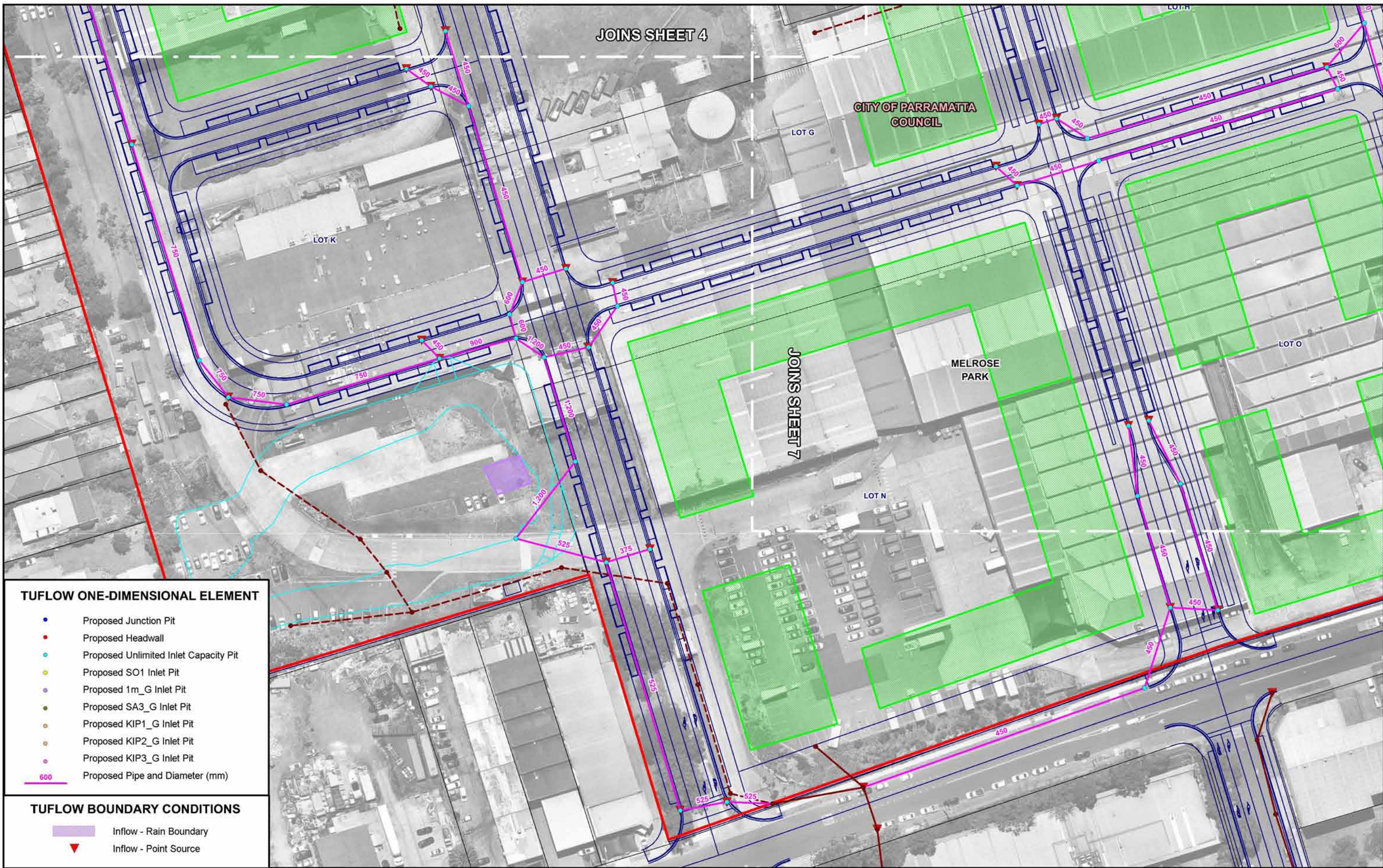


LEGEND

LGA Boundary	Existing Drainage Network to Remain	Developed Area
Proposed VRS Development Site	Existing Drainage Network to be Removed	Proposed Detention Basin
Proposed PP Development Site		
Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)		

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 1
(Sheet 5 of 7)
**TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS**

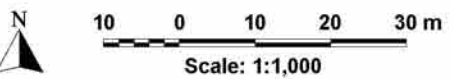


TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Junction Pit
- Proposed Headwall
- Proposed Unlimited Inlet Capacity Pit
- Proposed SO1 Inlet Pit
- Proposed 1m_G Inlet Pit
- Proposed SA3_G Inlet Pit
- Proposed KIP1_G Inlet Pit
- Proposed KIP2_G Inlet Pit
- Proposed KIP3_G Inlet Pit
- 600 Proposed Pipe and Diameter (mm)

TUFLOW BOUNDARY CONDITIONS

- ▭ Inflow - Rain Boundary
- ▴ Inflow - Point Source



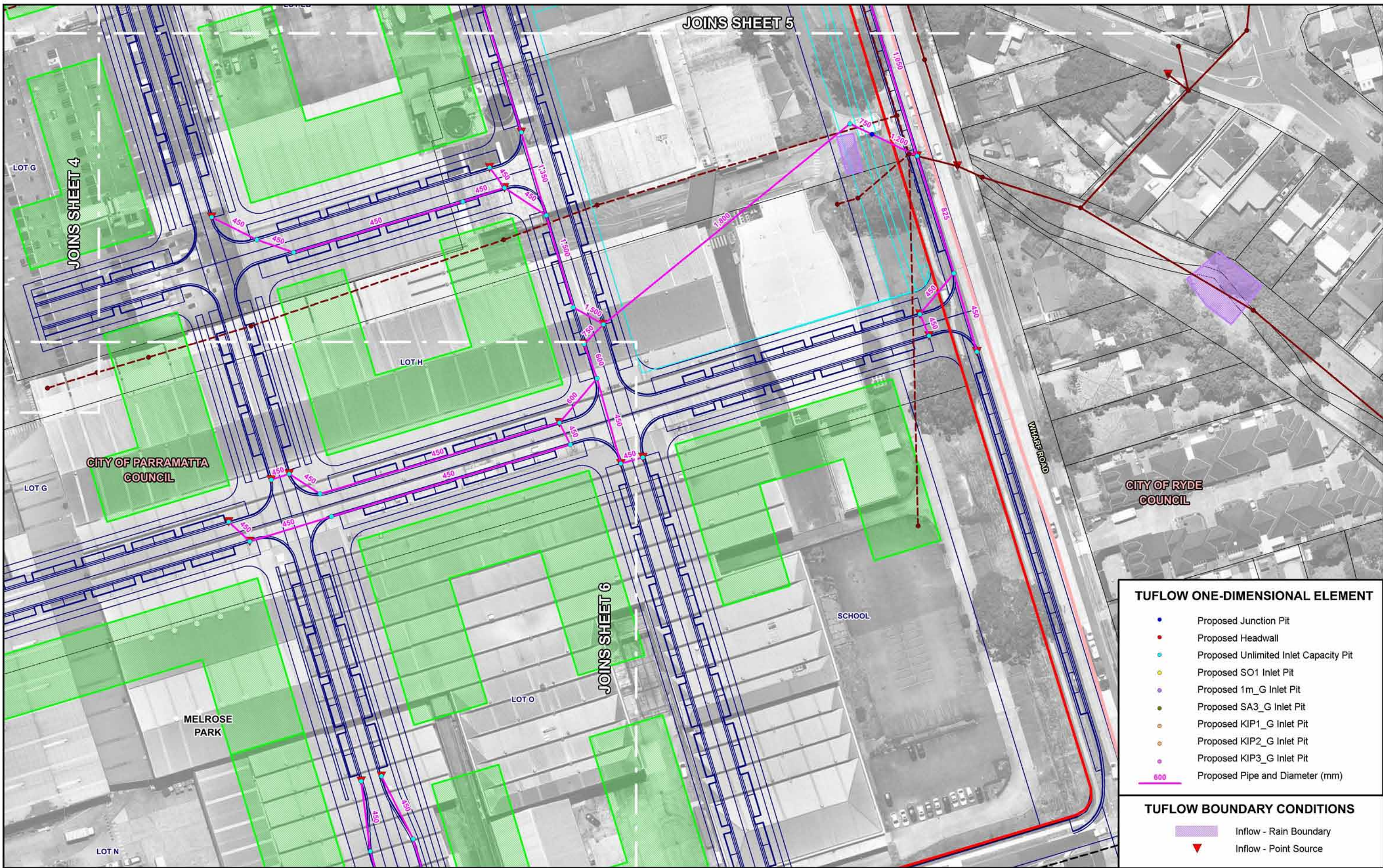
LEGEND

- ▭ LGA Boundary
- ▭ Proposed VRS Development Site
- ▭ Proposed PP Development Site
- ▭ Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)
- Existing Drainage Network to Remain
- - - Existing Drainage Network to be Removed

MELROSE PARK STORMWATER DRAINAGE STRATEGY

- ▭ Developed Area
- ▭ Proposed Detention Basin

Figure 1
(Sheet 6 of 7)
**TUFLOW MODEL LAYOUT
POST-VRS AND PP DEVELOPMENT CONDITIONS**



JOINS SHEET 5

JOINS SHEET 4

JOINS SHEET 6

TUFLOW ONE-DIMENSIONAL ELEMENT

- Proposed Junction Pit
- Proposed Headwall
- Proposed Unlimited Inlet Capacity Pit
- Proposed SO1 Inlet Pit
- Proposed 1m_G Inlet Pit
- Proposed SA3_G Inlet Pit
- Proposed KIP1_G Inlet Pit
- Proposed KIP2_G Inlet Pit
- Proposed KIP3_G Inlet Pit
- 600 Proposed Pipe and Diameter (mm)

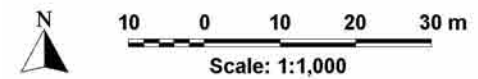
TUFLOW BOUNDARY CONDITIONS

- ▭ Inflow - Rain Boundary
- ▴ Inflow - Point Source

LEGEND

- ▭ LGA Boundary
- ▭ Proposed VRS Development Site
- ▭ Proposed PP Development Site
- ▭ Proposed MP Masterplan Design and Finished Road Level Contours (0.1m Interval)
- ▭ Existing Drainage Network to Remain
- ▭ Existing Drainage Network to be Removed
- ▭ Developed Area
- ▭ Proposed Detention Basin

MELROSE PARK STORMWATER DRAINAGE STRATEGY





Indicative Depth of Inundation (m)

< 0.05
0.05 to 0.10
0.10 to 0.20
0.20 to 0.30
0.30 to 0.40
0.40 to 0.50
0.50 to 0.60
0.60 to 0.70
0.70 to 0.80
0.80 to 0.90
0.90 to 1.00
> 1.00

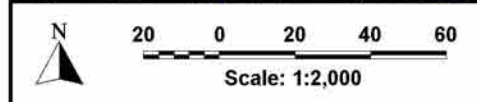
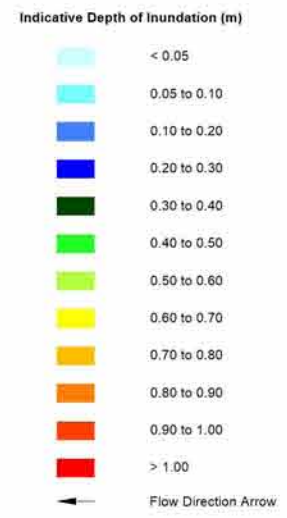
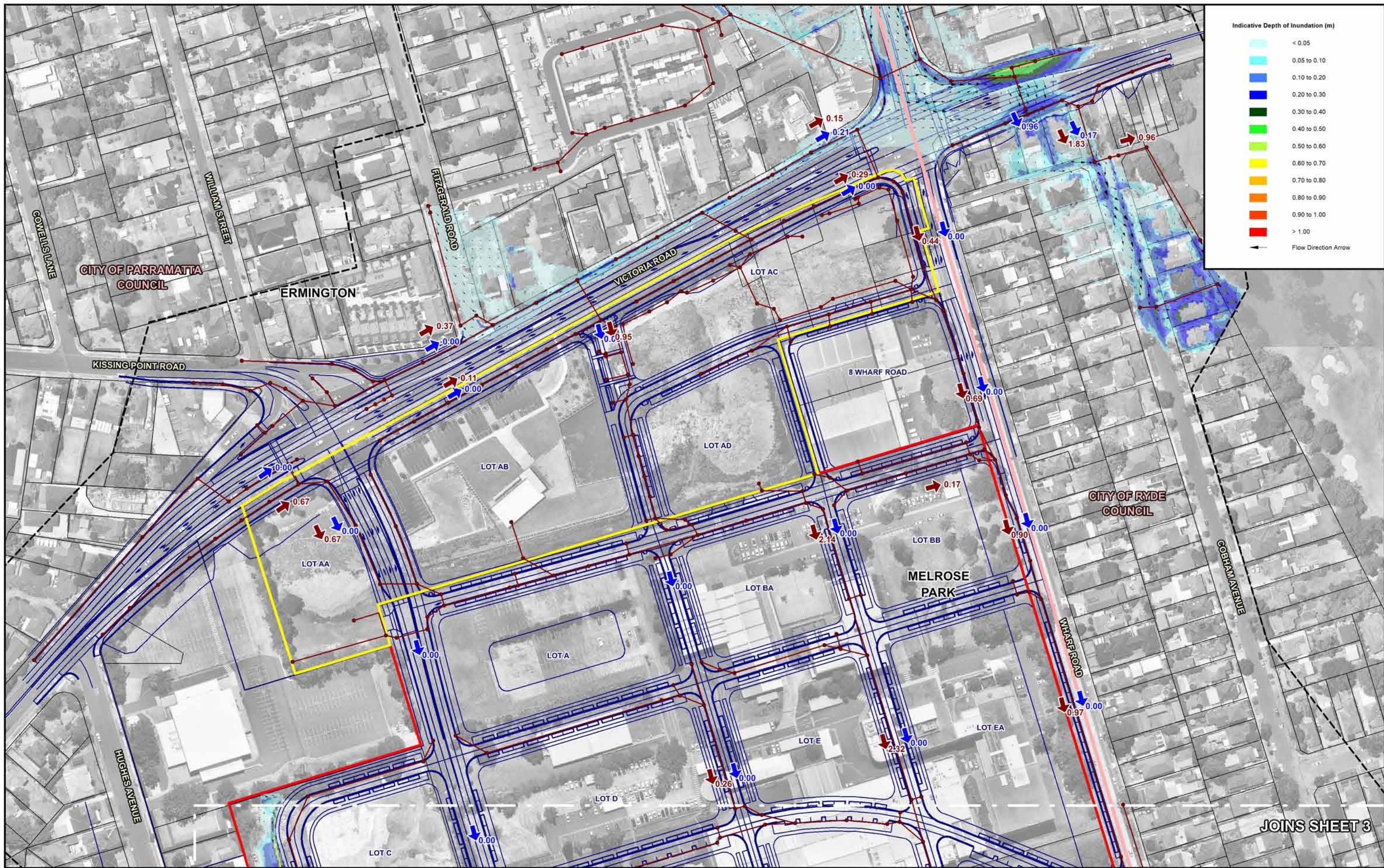
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Scale: 1:8,000



Figure 2
 (Sheet 1 of 3)
 INDICATIVE EXTENT AND DEPTH OF INUNDATION
 POST-VRS AND PP DEVELOPMENT CONDITIONS - 5% AEP



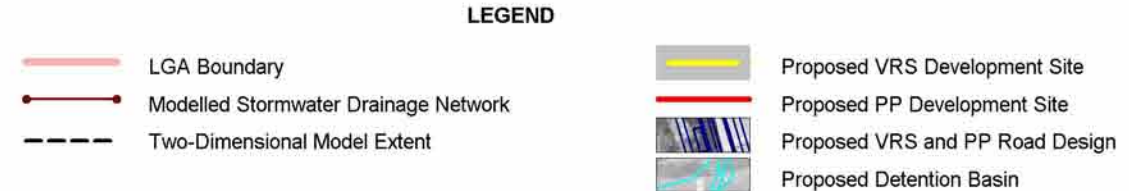
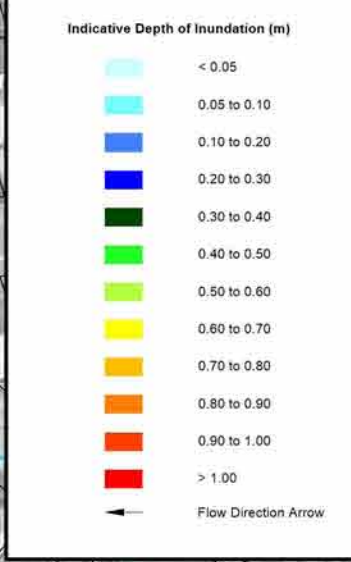
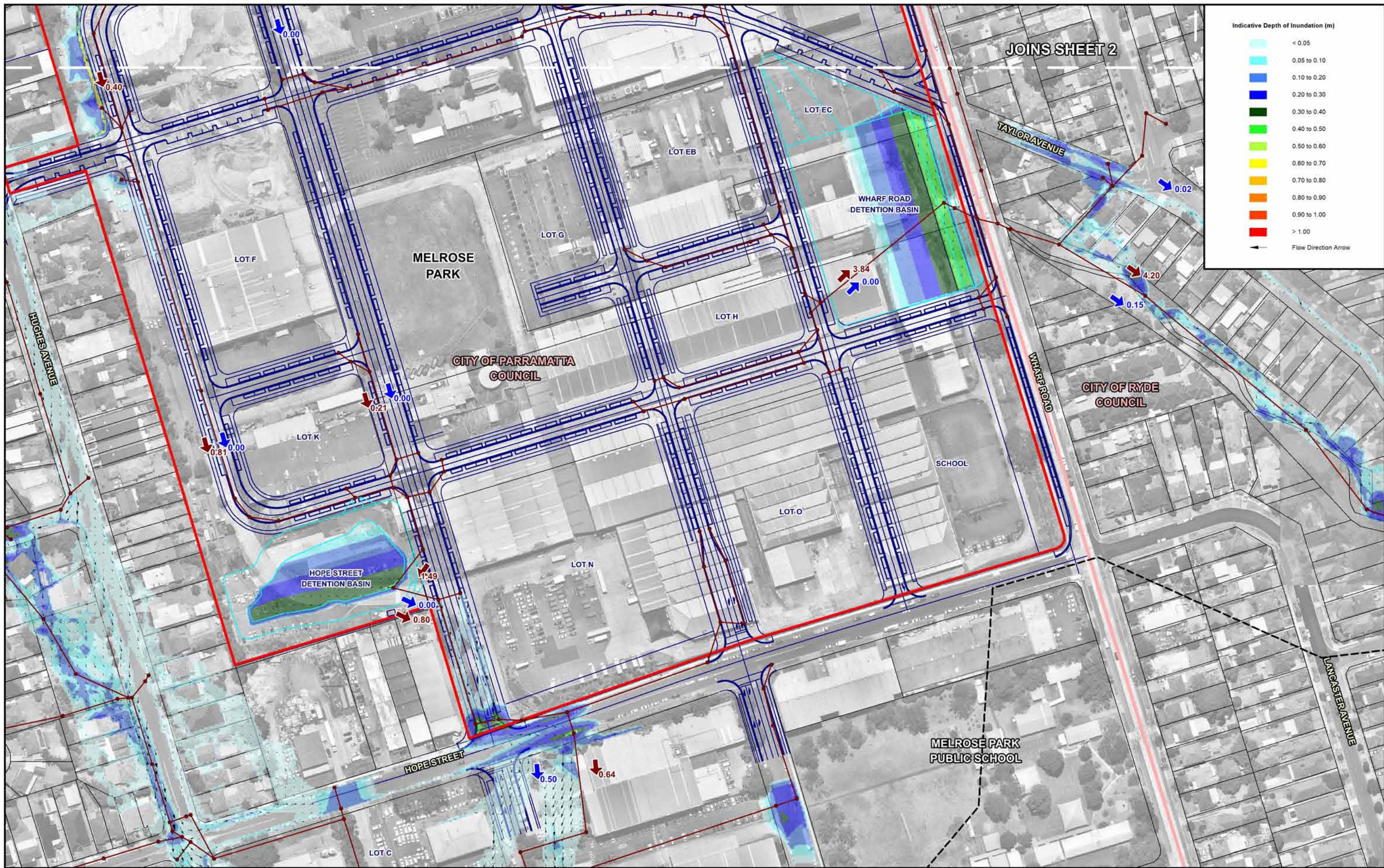
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

- MELROSE PARK STORMWATER DRAINAGE STRATEGY**
- ← 1.05 Peak Piped Flow (m^3/s)
 - ← 0.00 Peak Overland Flow (m^3/s)

INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT CONDITIONS - 5% AEP

Figure 2
(Sheet 2 of 3)

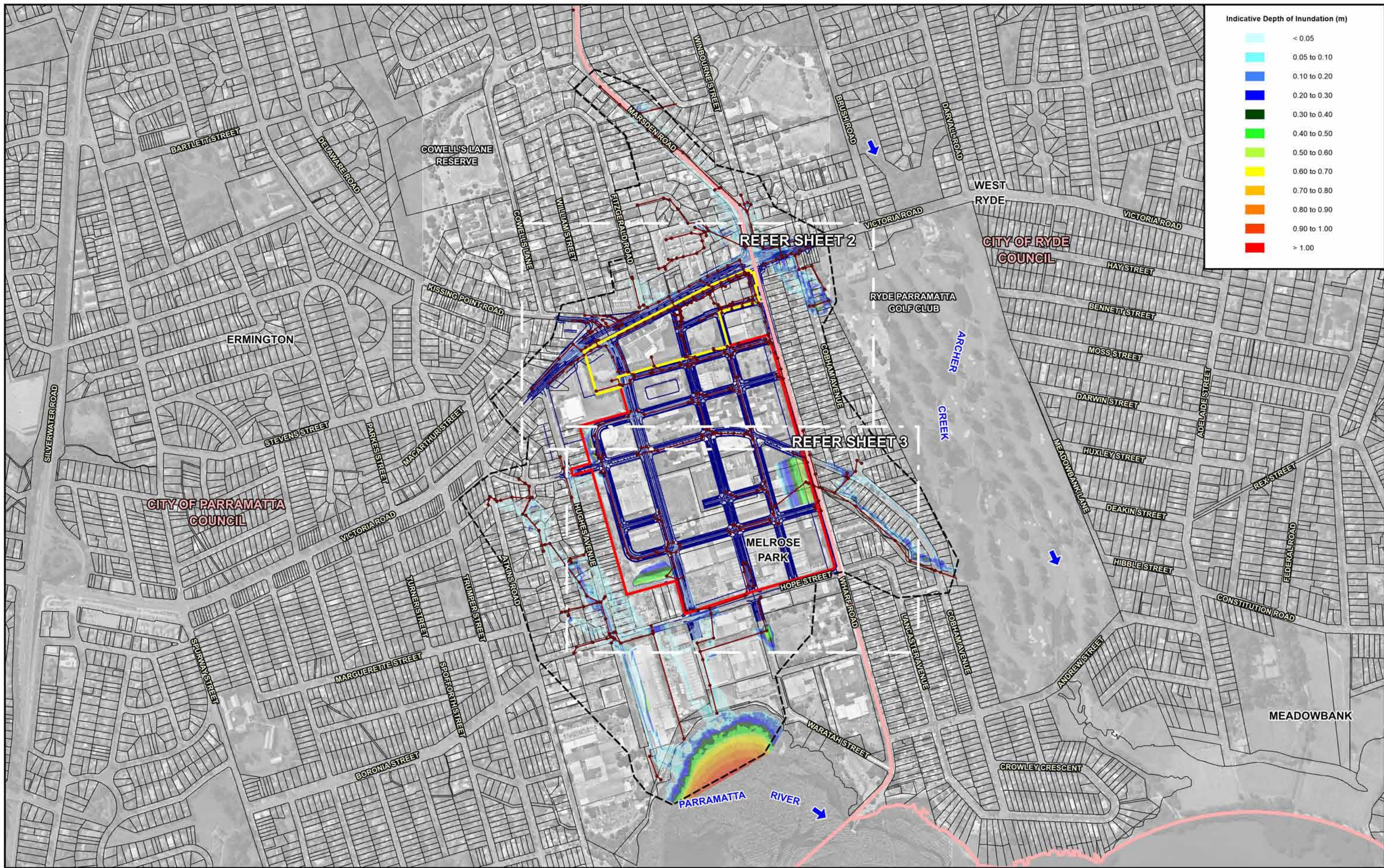
JOINS SHEET 3



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INDICATIVE EXTENT AND DEPTH OF INUNDATION POST-VRS AND PP DEVELOPMENT CONDITIONS - 5% AEP

Figure 2 (Sheet 3 of 3)



Indicative Depth of Inundation (m)

< 0.05
0.05 to 0.10
0.10 to 0.20
0.20 to 0.30
0.30 to 0.40
0.40 to 0.50
0.50 to 0.60
0.60 to 0.70
0.70 to 0.80
0.80 to 0.90
0.90 to 1.00
> 1.00

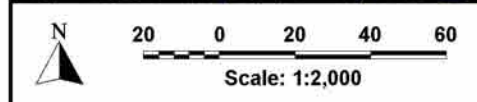
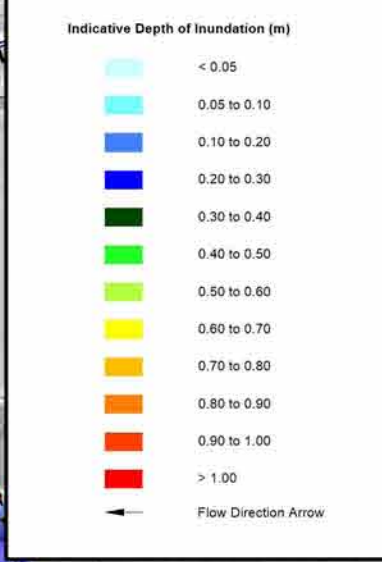
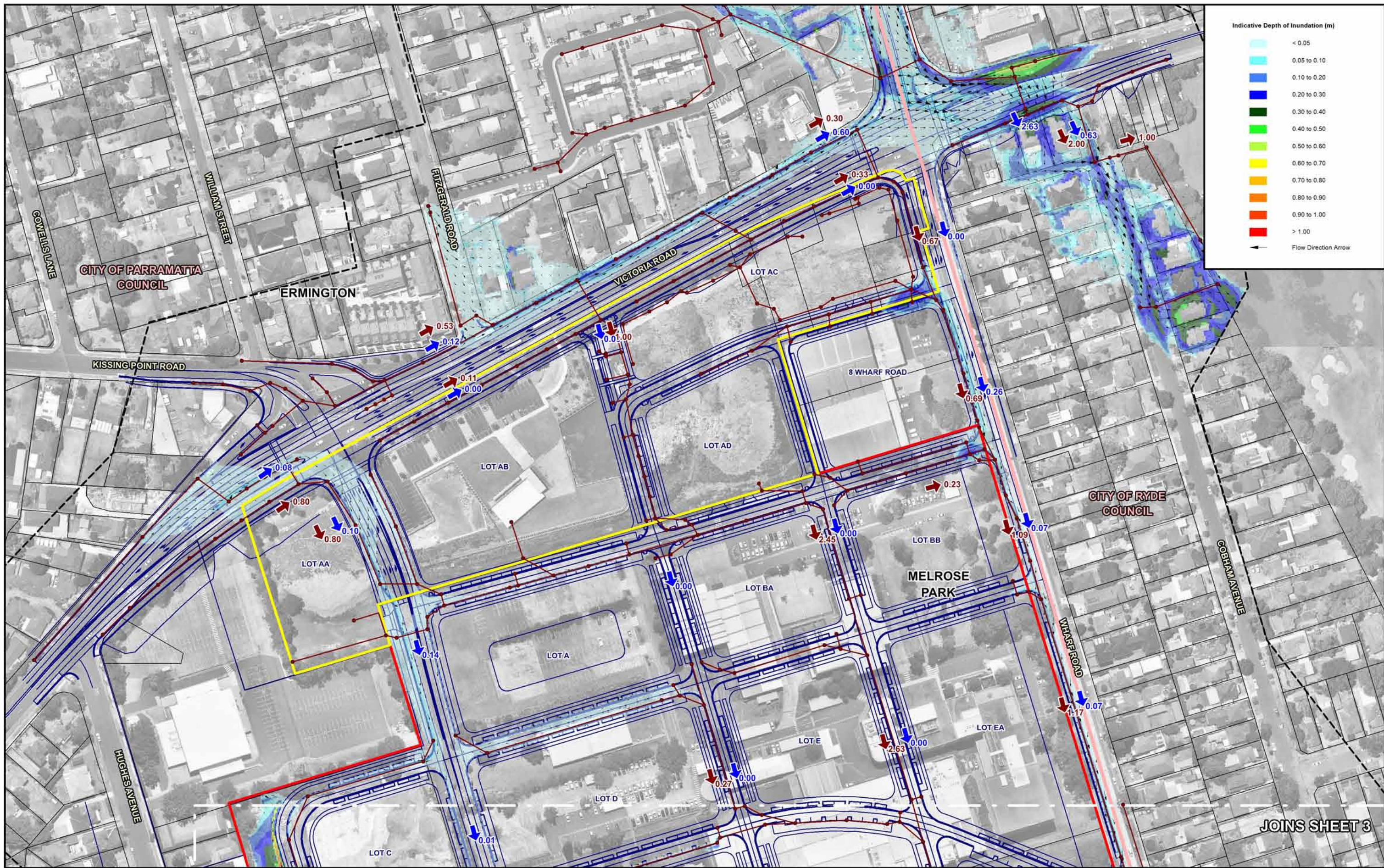
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

Scale: 1:8,000



MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 3
 (Sheet 1 of 3)
 INDICATIVE EXTENT AND DEPTH OF INUNDATION
 POST-VRS AND PP DEVELOPMENT CONDITIONS - 1% AEP



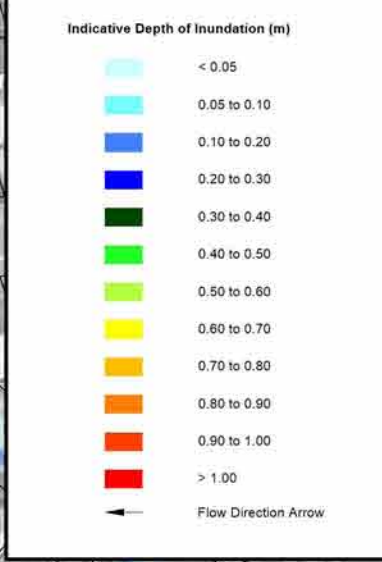
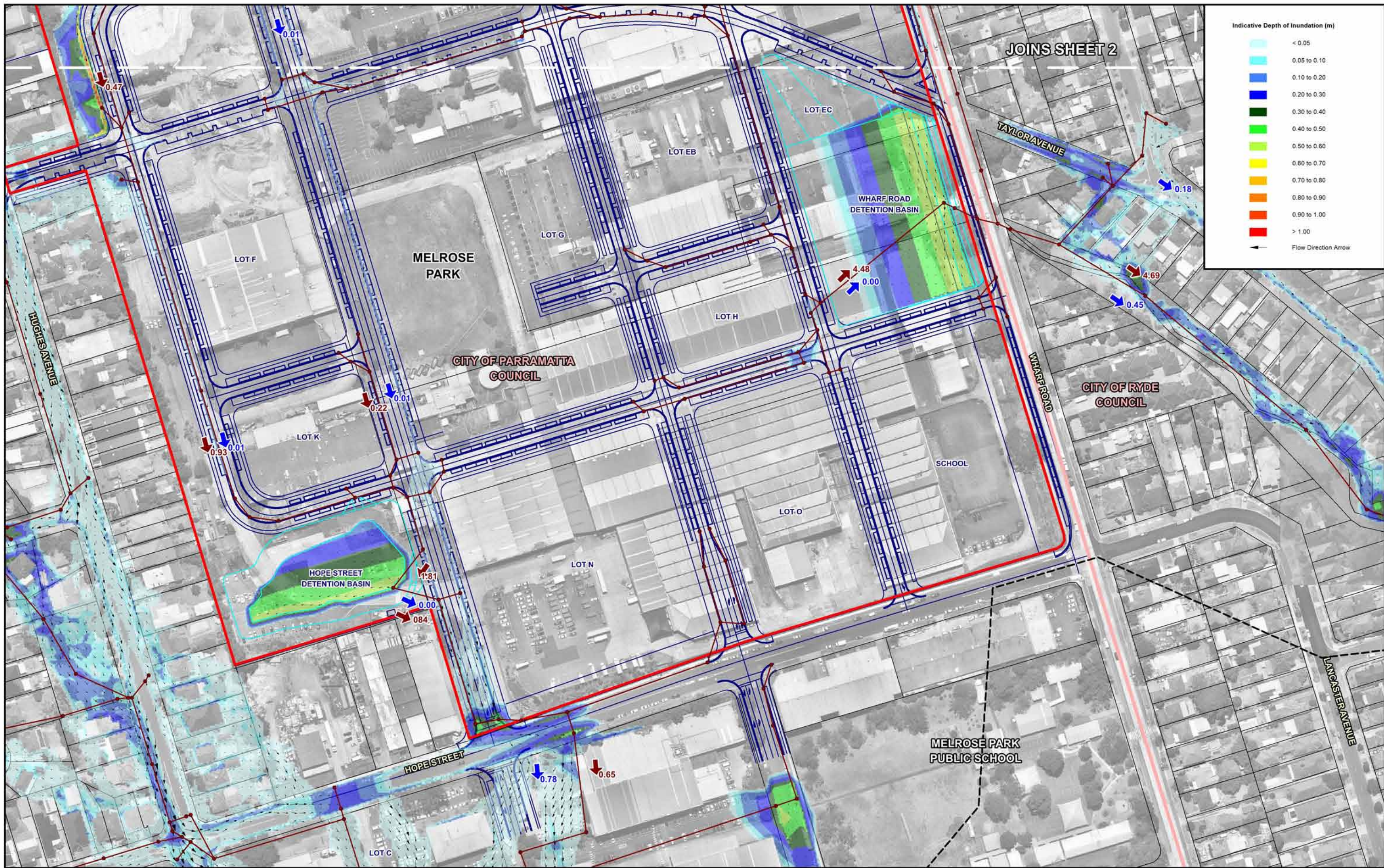
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

- MELROSE PARK STORMWATER DRAINAGE STRATEGY**
- ← 0.99 Peak Piped Flow (m³/s)
 - ← 0.00 Peak Overland Flow (m³/s)

INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT CONDITIONS - 1% AEP

Figure 3
(Sheet 2 of 3)

JOINS SHEET 3



JOINS SHEET 2



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LEGEND

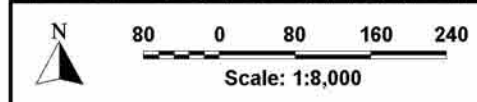
- LGA Boundary
- Modelled Stormwater Drainage Network
- Two-Dimensional Model Extent
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed VRS and PP Road Design
- Proposed Detention Basin

MELROSE PARK STORMWATER DRAINAGE STRATEGY

- ← 0.99 Peak Piped Flow (m³/s)
- ← 0.00 Peak Overland Flow (m³/s)

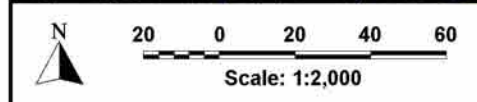
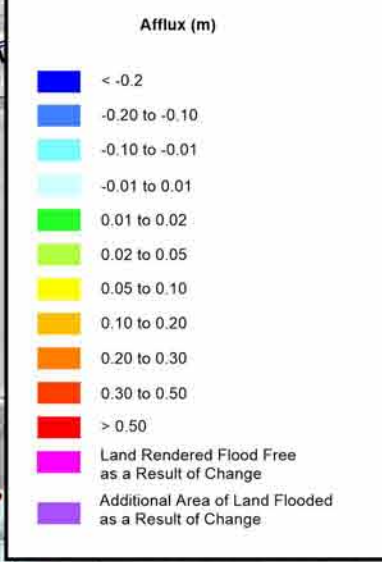
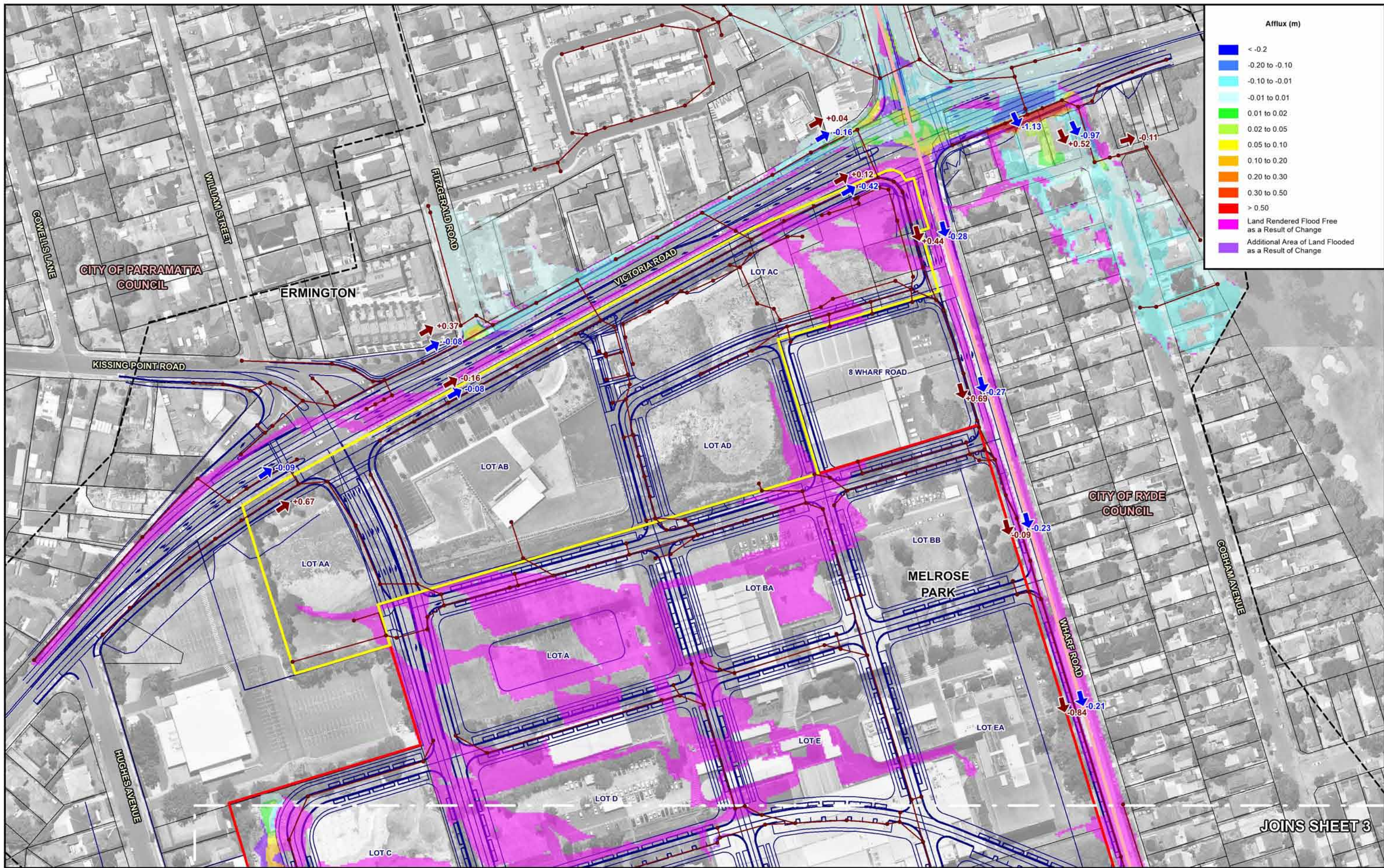
Figure 3
(Sheet 3 of 3)

INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT CONDITIONS - 1% AEP



Note
 1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.

- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - - - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design



LEGEND

- LGA Boundary
- Modelled Stormwater Drainage Network
- Two-Dimensional Model Extent
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed VRS and PP Road Design

Note
1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.

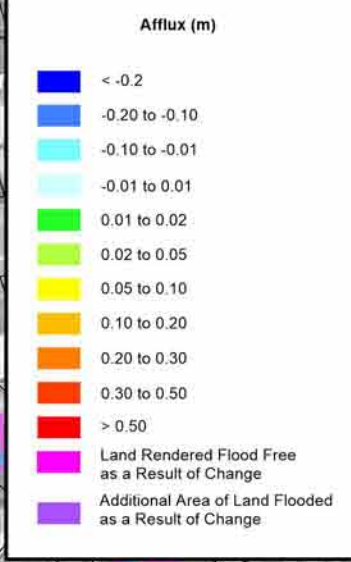
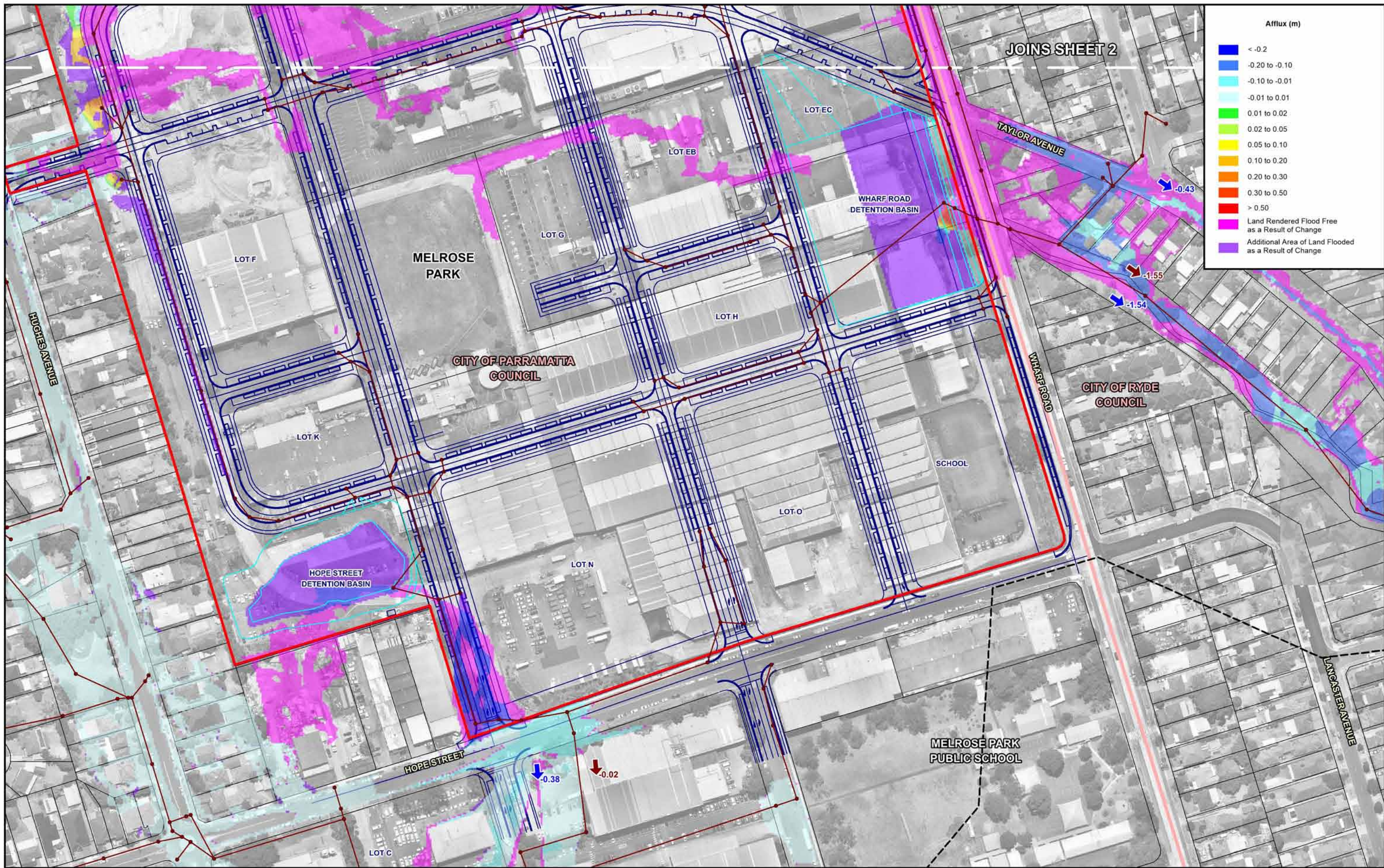
MELROSE PARK STORMWATER DRAINAGE STRATEGY

- +0.29 Change in Peak Piped Flow (m^3/s)
- 0.36 Change in Peak Overland Flow (m^3/s)

IMPACT OF PROPOSED VRS AND PP DEVELOPMENT ON FLOOD BEHAVIOUR 5% AEP

Figure 4
(Sheet 2 of 3)

JOINS SHEET 3



JOINS SHEET 2



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Note
1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.

LEGEND

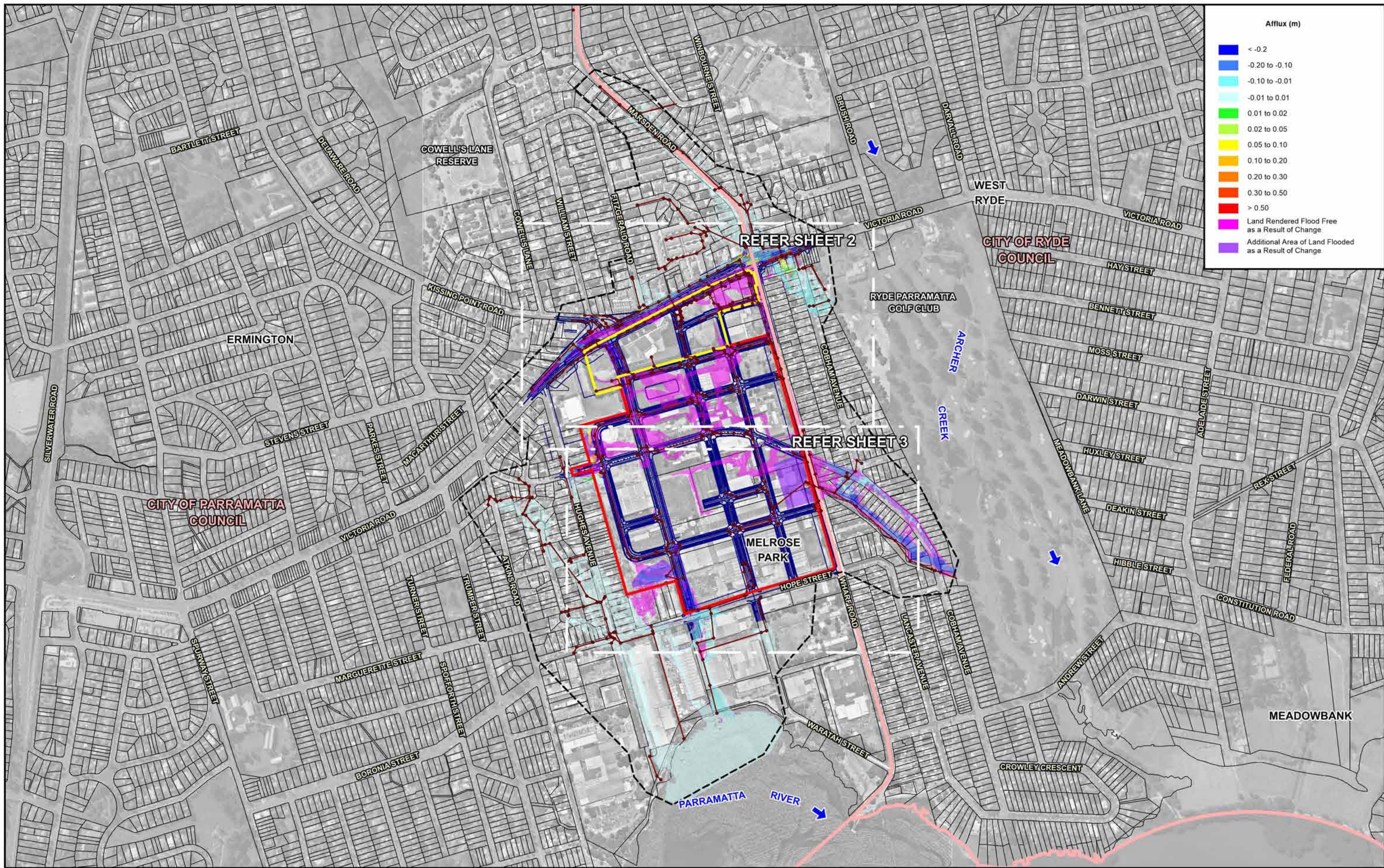
- LGA Boundary
- Modelled Stormwater Drainage Network
- Two-Dimensional Model Extent
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed VRS and PP Road Design
- Proposed Detention Basin

- ↖ +0.29 Change in Peak Piped Flow (m³/s)
- ↖ -0.36 Change in Peak Overland Flow (m³/s)

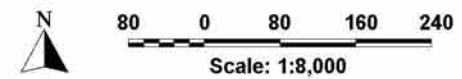
MELROSE PARK STORMWATER DRAINAGE STRATEGY

IMPACT OF PROPOSED VRS AND PP DEVELOPMENT ON FLOOD BEHAVIOUR
5% AEP

Figure 4
(Sheet 3 of 3)



Afflux (m)	
Dark Blue	< -0.2
Blue	-0.20 to -0.10
Cyan	-0.10 to -0.01
Light Cyan	-0.01 to 0.01
Green	0.01 to 0.02
Light Green	0.02 to 0.05
Yellow	0.05 to 0.10
Orange	0.10 to 0.20
Red-Orange	0.20 to 0.30
Red	0.30 to 0.50
Red	> 0.50
Pink	Land Rendered Flood Free as a Result of Change
Purple	Additional Area of Land Flooded as a Result of Change

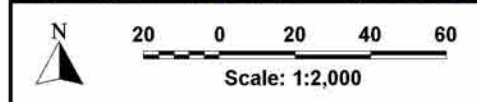
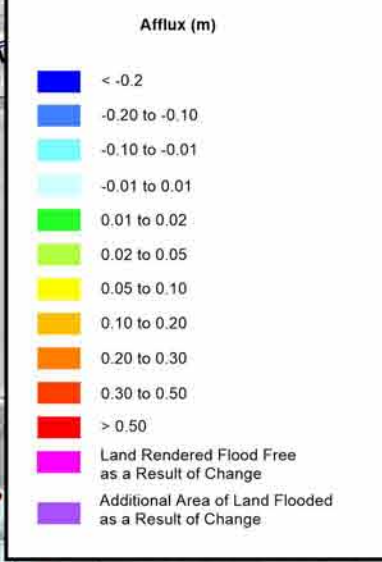
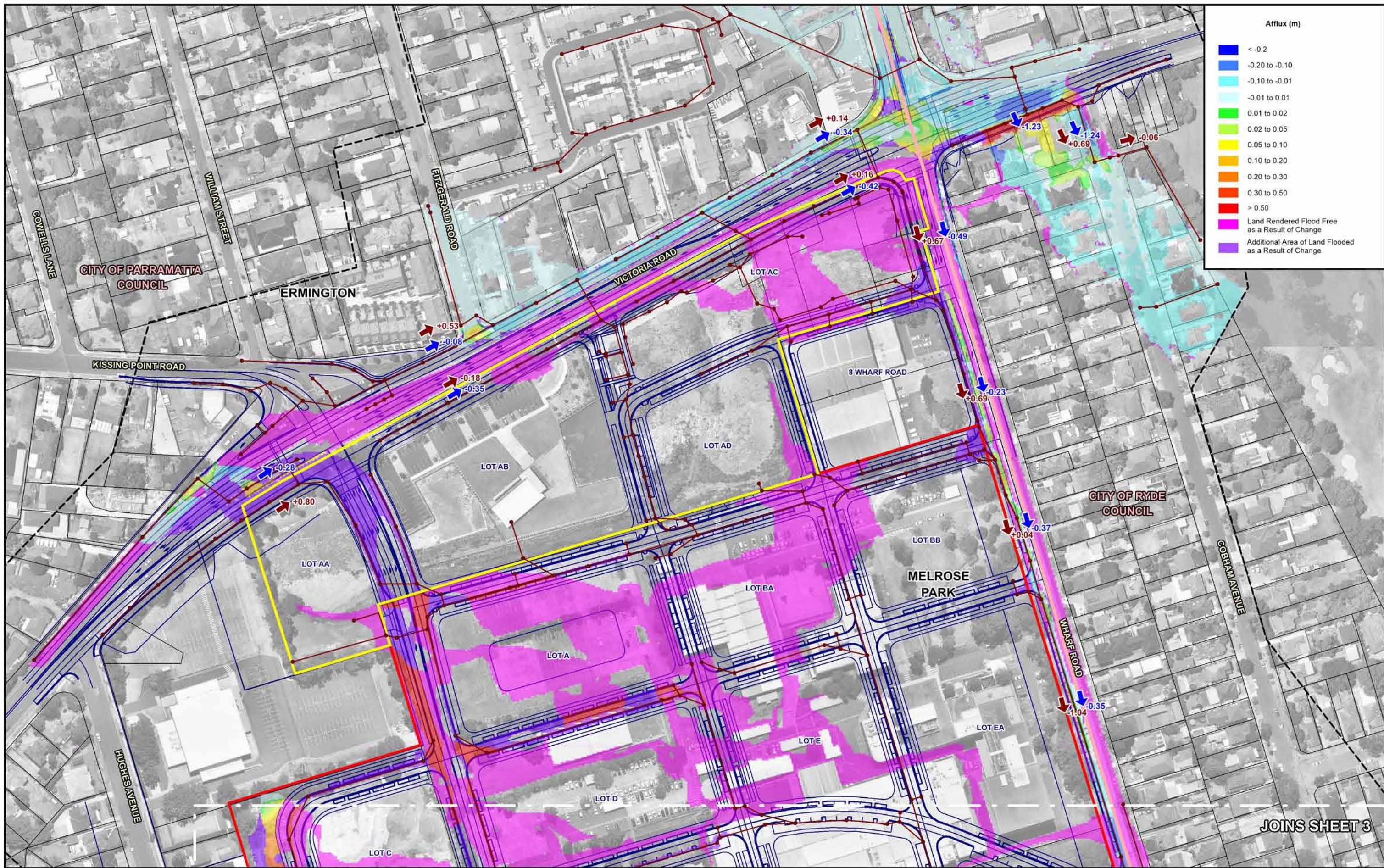


- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - - - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

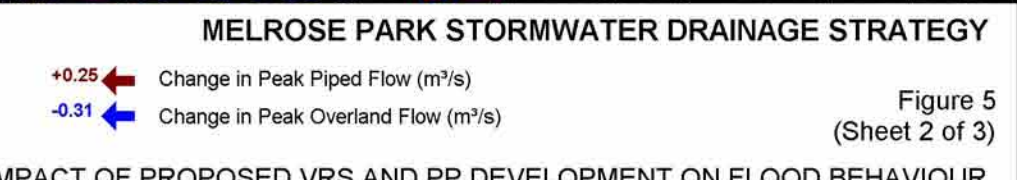
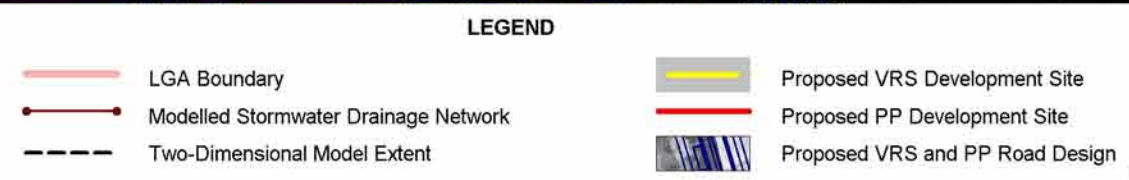
Note
 1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.

MELROSE PARK STORMWATER DRAINAGE STRATEGY

Figure 5
 (Sheet 1 of 3)
 IMPACT OF PROPOSED VRS AND PP DEVELOPMENT ON FLOOD BEHAVIOUR
 1% AEP



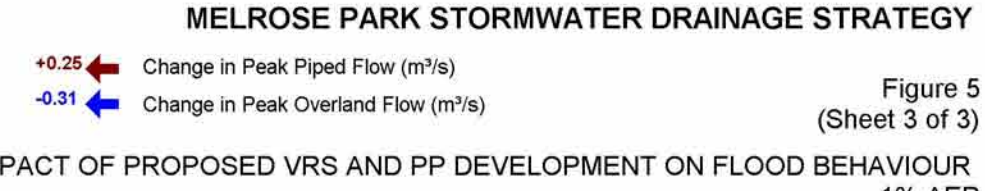
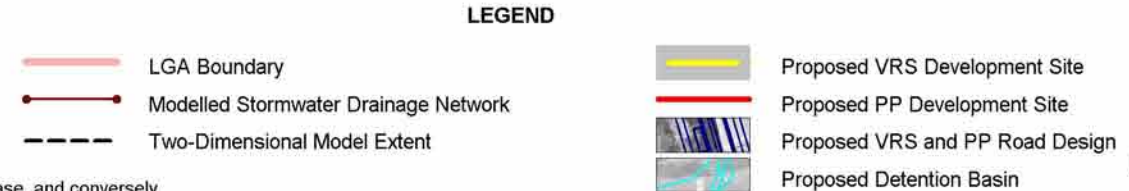
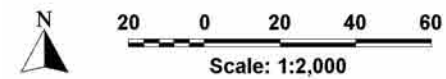
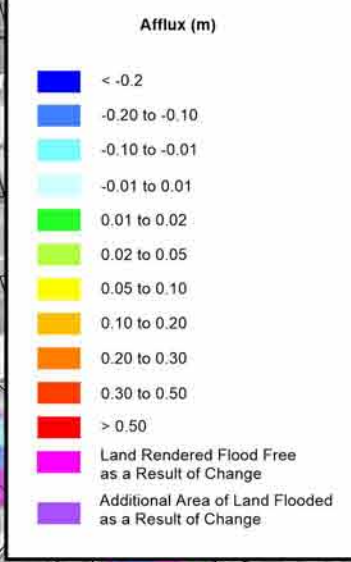
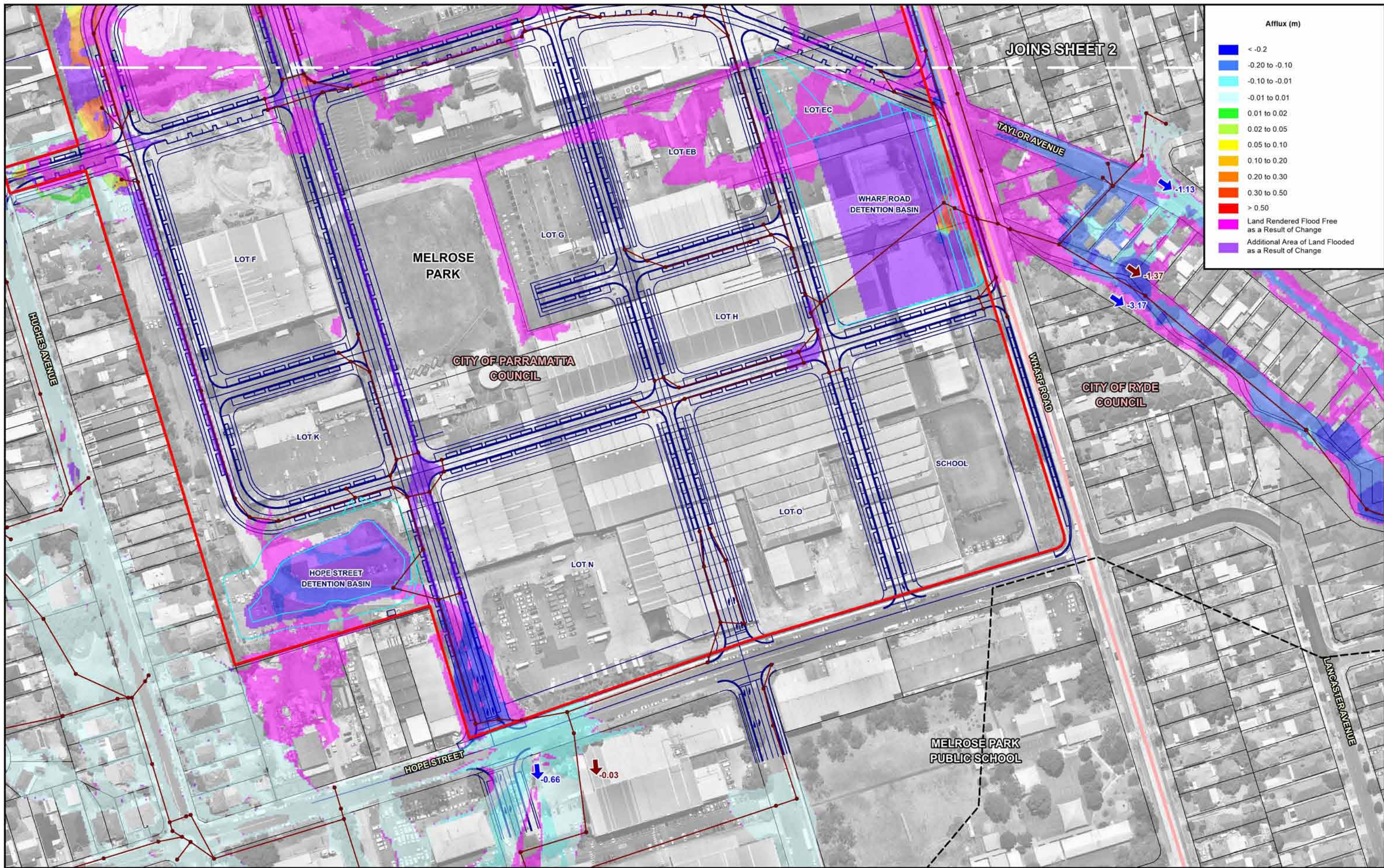
Note
 1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.



IMPACT OF PROPOSED VRS AND PP DEVELOPMENT ON FLOOD BEHAVIOUR
 1% AEP

Figure 5
 (Sheet 2 of 3)

JOINS SHEET 3



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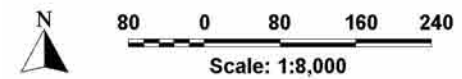
Note
1. A positive value represents an increase, and conversely a negative value represents a decrease in peak flow when compared to baseline conditions.

Figure 5
(Sheet 3 of 3)
MELROSE PARK STORMWATER DRAINAGE STRATEGY
IMPACT OF PROPOSED VRS AND PP DEVELOPMENT ON FLOOD BEHAVIOUR 1% AEP



Indicative Depth of Inundation (m)

<math>< 0.05</math>
0.05 to 0.10
0.10 to 0.20
0.20 to 0.30
0.30 to 0.40
0.40 to 0.50
0.50 to 0.60
0.60 to 0.70
0.70 to 0.80
0.80 to 0.90
0.90 to 1.00
> 1.00



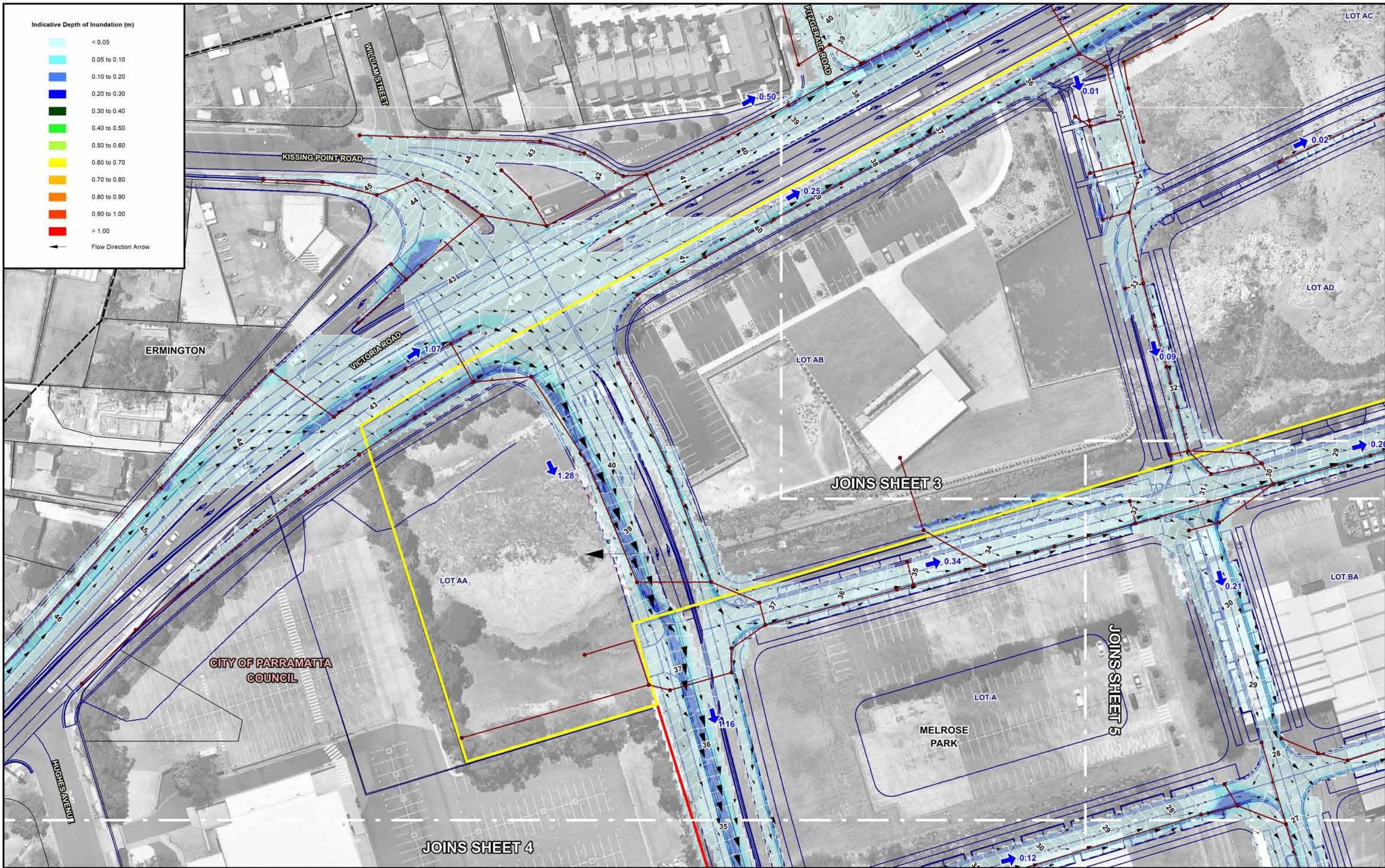
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

MELROSE PARK STORMWATER DRAINAGE STRATEGY



Figure 6
(Sheet 1 of 7)

INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP



MELROSE PARK STORMWATER DRAINAGE STRATEGY

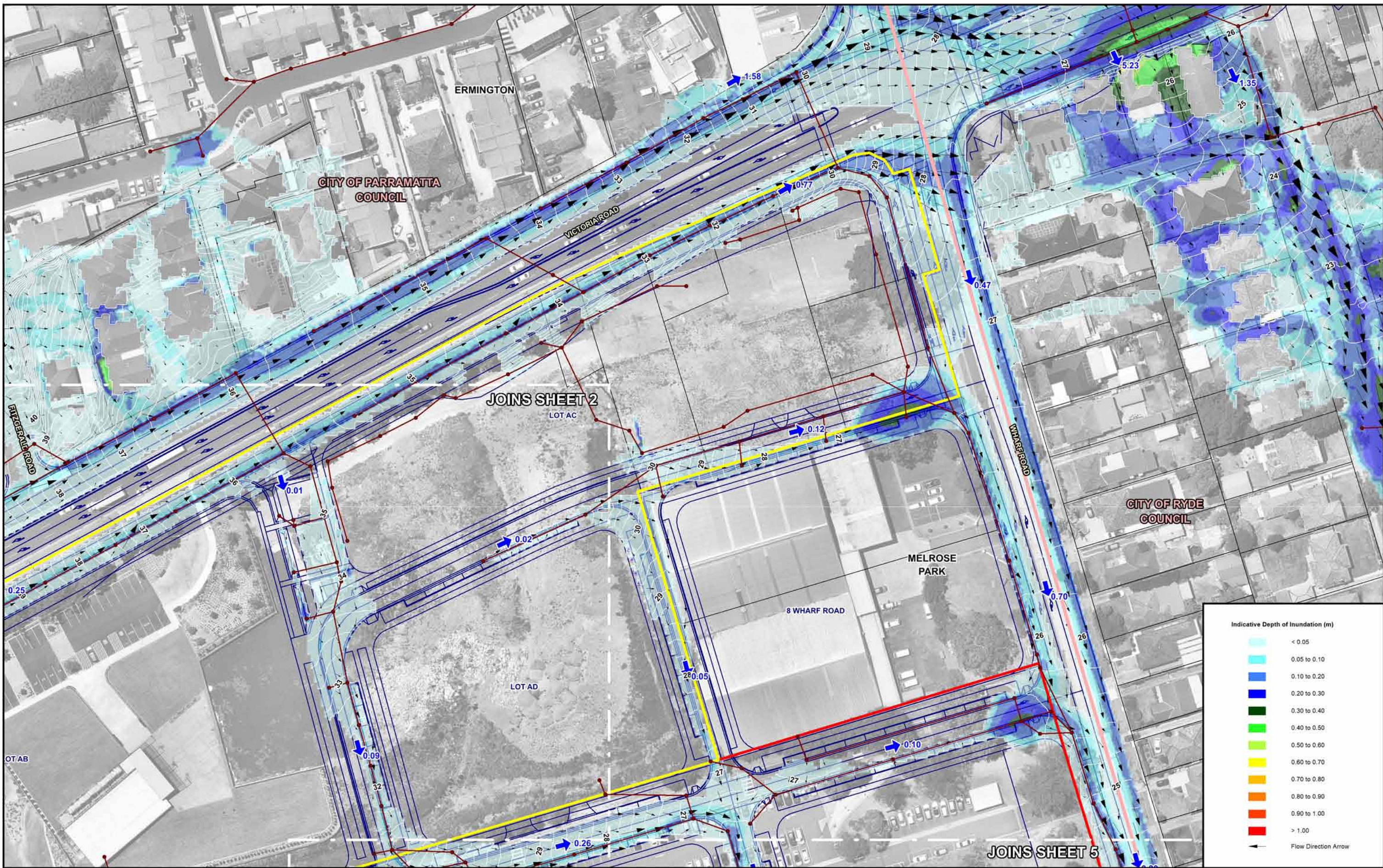
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LEGEND

- LGA Boundary
- Modelled Stormwater Drainage Network
- Two-Dimensional Model Extent
- Water Surface Elevation Contours (0.2m Interval)
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed VRS and PP Road Design
- Peak Overland Flow (m^3/s)

Figure 6 (Sheet 2 of 7)

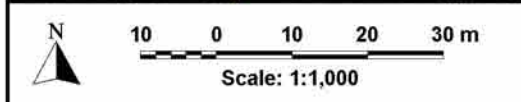
INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP



Indicative Depth of Inundation (m)

< 0.05
0.05 to 0.10
0.10 to 0.20
0.20 to 0.30
0.30 to 0.40
0.40 to 0.50
0.50 to 0.60
0.60 to 0.70
0.70 to 0.80
0.80 to 0.90
0.90 to 1.00
> 1.00

Flow Direction Arrow



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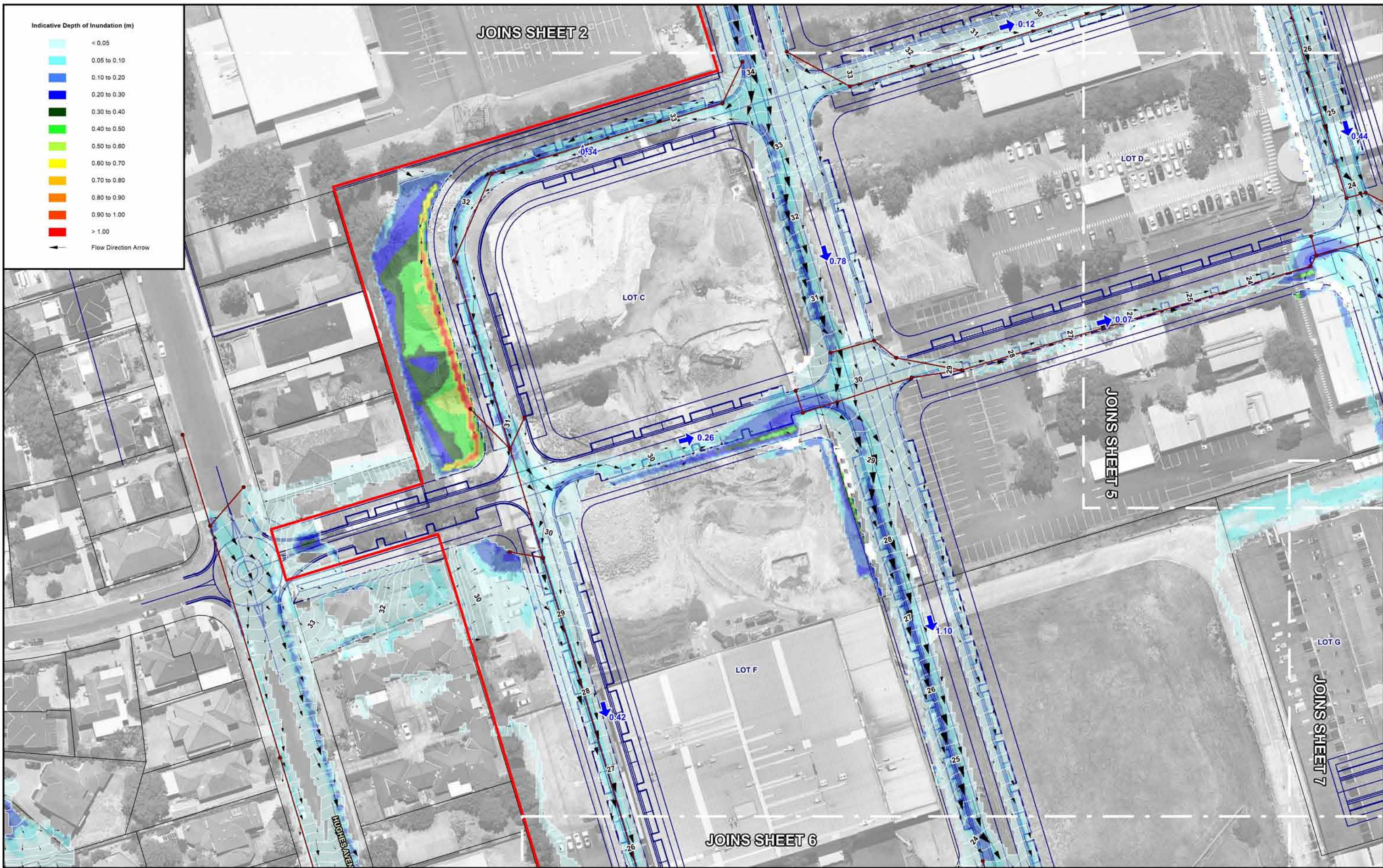
- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Water Surface Elevation Contours (0.2m Interval)
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design

MELROSE PARK STORMWATER DRAINAGE STRATEGY

1.58 ← Peak Overland Flow (m³/s)

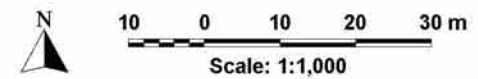
INDICATIVE EXTENT AND DEPTH OF INUNDATION POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP

Figure 6 (Sheet 3 of 7)



Indicative Depth of Inundation (m)

- < 0.05
- 0.05 to 0.10
- 0.10 to 0.20
- 0.20 to 0.30
- 0.30 to 0.40
- 0.40 to 0.50
- 0.50 to 0.60
- 0.60 to 0.70
- 0.70 to 0.80
- 0.80 to 0.90
- 0.90 to 1.00
- > 1.00
- Flow Direction Arrow



- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - - - Two-Dimensional Model Extent
 - Water Surface Elevation Contours (0.2m Interval)

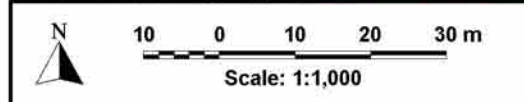
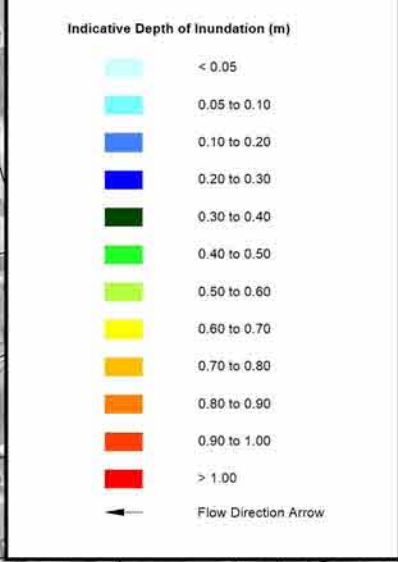
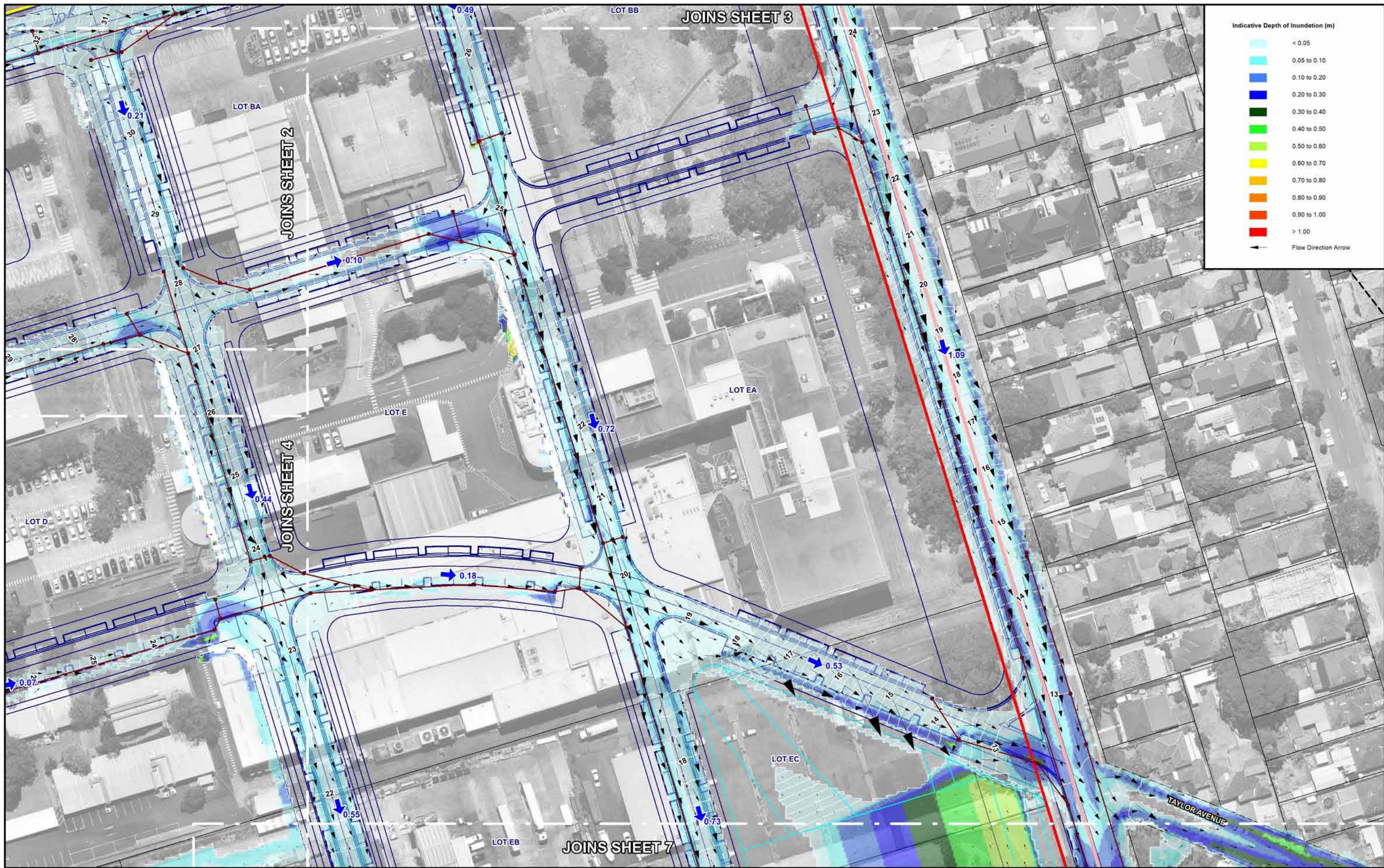
- Proposed VRS Development Site
- Proposed PP Development Site
- ▨ Proposed VRS and PP Road Design

1.58 ← Peak Overland Flow (m³/s)

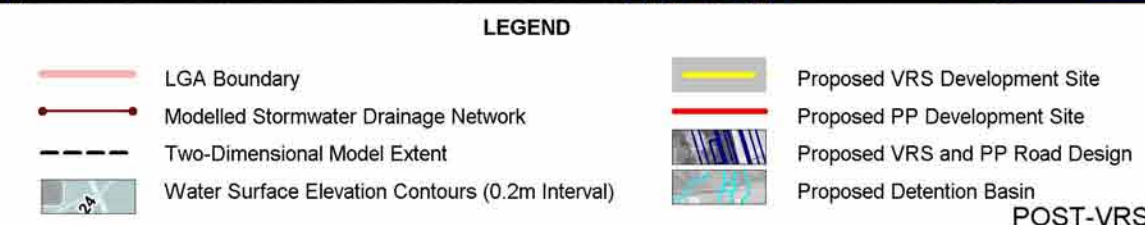
MELROSE PARK STORMWATER DRAINAGE STRATEGY



Figure 6
(Sheet 4 of 7)
**INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP**



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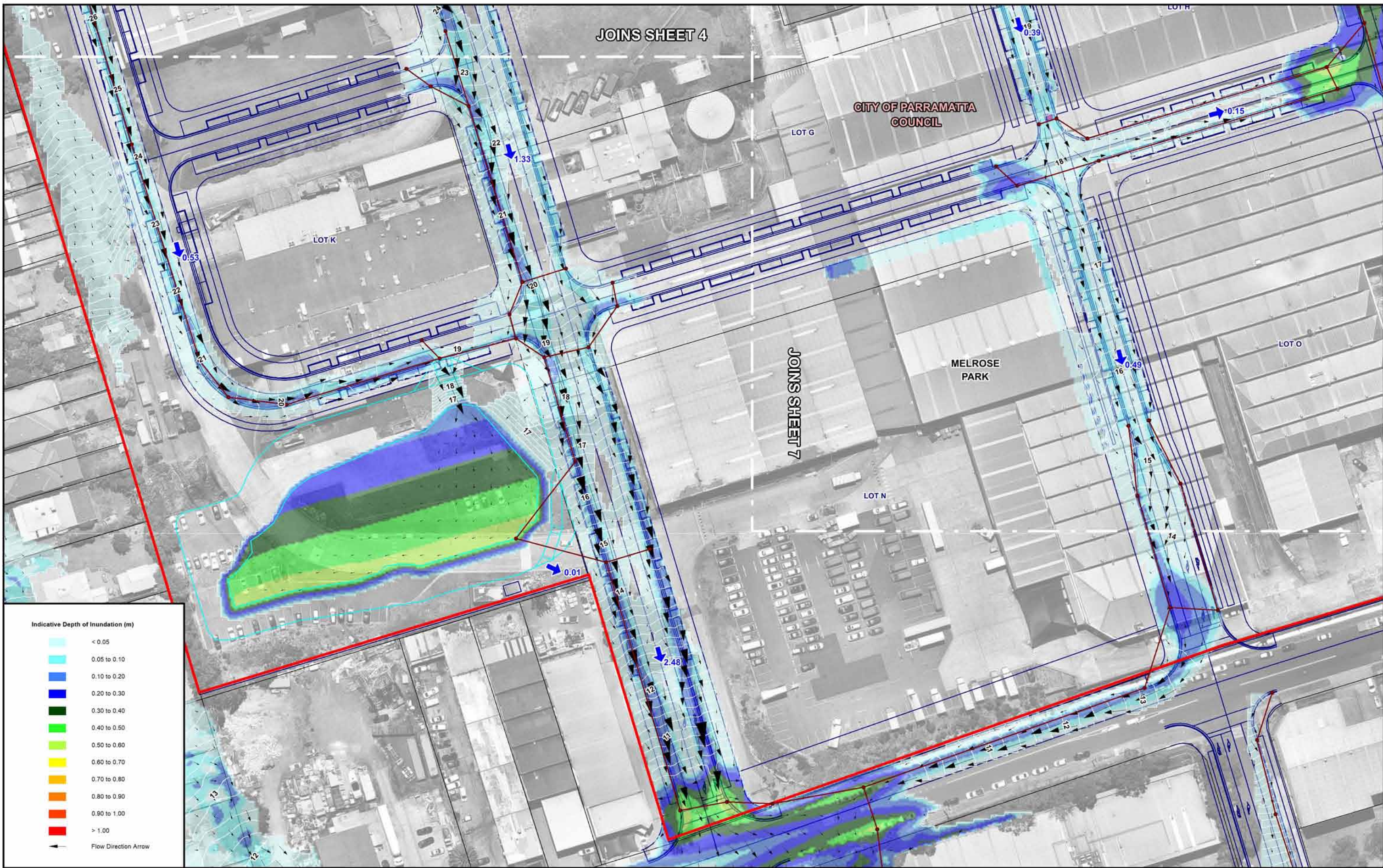


MELROSE PARK STORMWATER DRAINAGE STRATEGY

1.58 ← Peak Overland Flow (m³/s)

INDICATIVE EXTENT AND DEPTH OF INUNDATION
POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP

Figure 6
(Sheet 5 of 7)



JOINS SHEET 4

CITY OF PARRAMATTA COUNCIL

JOINS SHEET 7

Indicative Depth of Inundation (m)

- < 0.05
- 0.05 to 0.10
- 0.10 to 0.20
- 0.20 to 0.30
- 0.30 to 0.40
- 0.40 to 0.50
- 0.50 to 0.60
- 0.60 to 0.70
- 0.70 to 0.80
- 0.80 to 0.90
- 0.90 to 1.00
- > 1.00
- Flow Direction Arrow

LEGEND

- LGA Boundary
- Modelled Stormwater Drainage Network
- - - Two-Dimensional Model Extent
- Water Surface Elevation Contours (0.2m Interval)
- Proposed VRS Development Site
- Proposed PP Development Site
- Proposed VRS and PP Road Design
- Proposed Detention Basin

MELROSE PARK STORMWATER DRAINAGE STRATEGY

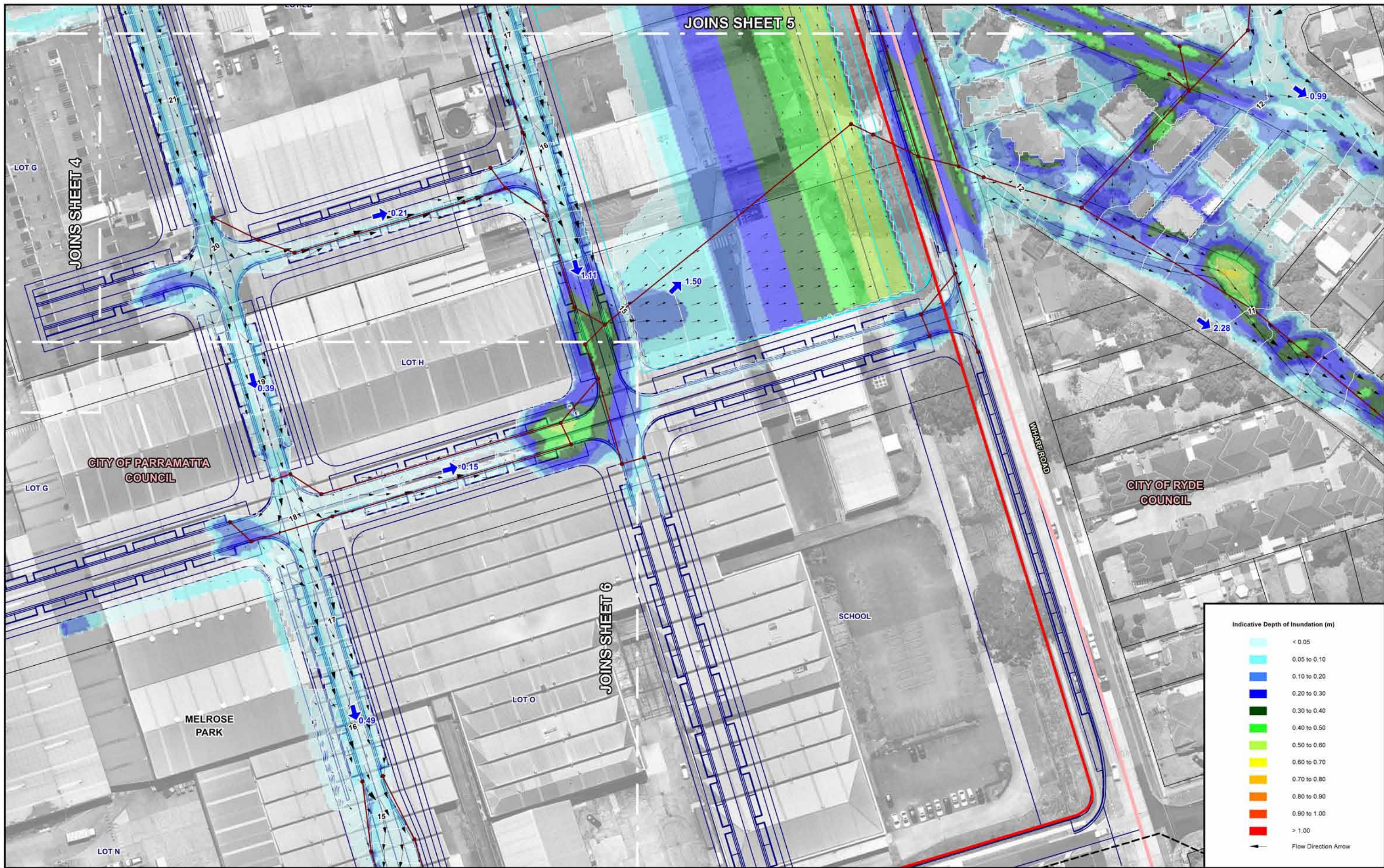
1.58 ← Peak Overland Flow (m³/s)

Scale: 1:1,000



Figure 6 (Sheet 6 of 7)

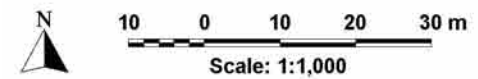
INDICATIVE EXTENT AND DEPTH OF INUNDATION POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP



Indicative Depth of Inundation (m)

	< 0.05
	0.05 to 0.10
	0.10 to 0.20
	0.20 to 0.30
	0.30 to 0.40
	0.40 to 0.50
	0.50 to 0.60
	0.60 to 0.70
	0.70 to 0.80
	0.80 to 0.90
	0.90 to 1.00
	> 1.00

← Flow Direction Arrow



- LEGEND**
- LGA Boundary
 - Modelled Stormwater Drainage Network
 - Two-Dimensional Model Extent
 - Water Surface Elevation Contours (0.2m Interval)
 - Proposed VRS Development Site
 - Proposed PP Development Site
 - Proposed VRS and PP Road Design
 - Proposed Detention Basin

MELROSE PARK STORMWATER DRAINAGE STRATEGY

1.58 ← Peak Overland Flow (m³/s)



INDICATIVE EXTENT AND DEPTH OF INUNDATION POST-VRS AND PP DEVELOPMENT AND COMPLETE BLOCKAGE CONDITIONS - 1% AEP

Figure 6
(Sheet 7 of 7)

ANNEXURE A

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block A				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.3297	ha	13,297	m ²	
Total Roof Area	0.4683	ha	4,683	m ²	
Area of Site draining to OSD Storage	0.851	ha	8,510	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.861	ha			
Area Bypassing Storage	0.4787	ha			
Area Bypassing / Residual Site Area	55.6%				Unacceptable - Exceeds 30% Satisfactory 30% Max
No. of Dwellings on Site	5				
Site Area per Dwelling	0.266	ha			
Roof Area per Dwelling	0.094	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		44%			
Adjusted SRDs		23	L/s/ha	39	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	325.78	m ³	Total Storage	526.56 m ³
Total Rainwater Tank Credits		0.29	m ³		0.27 m ³
Storage Volume				Total	526.29 m ³
Storage Volume	Ext Detention Storage	325.49	m ³	Flood Detention Storage	200.81 m ³
OSD Discharges	Primary Outlet	31.02	L/s	Secondary Outlet	51.67 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10		Satisfactory	Satisfactory 0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		122	mm	Satisfactory	Satisfactory 154 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block BA				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.8441	ha	8,441	m ²	
Total Roof Area	0.3093	ha	3,093	m ²	
Area of Site draining to OSD Storage	0.4988	ha	4,988	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.535	ha			
Area Bypassing Storage	0.3453	ha			
Area Bypassing / Residual Site Area	64.6%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	3				Satisfactory
Site Area per Dwelling	0.281	ha			
Roof Area per Dwelling	0.103	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	35%				
Adjusted SRDs	21	L/s/ha		21	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	206.80	m ³	Total Storage	334.26 m ³
Total Rainwater Tank Credits		0.14	m ³		0.13 m ³
Storage Volume				Total	334.13 m ³
Storage Volume	Ext Detention Storage	206.66	m ³	Flood Detention Storage	127.47 m ³
OSD Discharges	Primary Outlet	17.41	L/s	Secondary Outlet	17.61 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	91	mm	Satisfactory	Satisfactory	90 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block BB				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.8204	ha	8,204	m ²	
Total Roof Area	0.2742	ha	2,742	m ²	
Area of Site draining to OSD Storage	0.4499	ha	4,499	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.546	ha			
Area Bypassing Storage	0.3705	ha			
Area Bypassing / Residual Site Area	67.8%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	3				Satisfactory
Site Area per Dwelling	0.273	ha			
Roof Area per Dwelling	0.091	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	32%				
Adjusted SRDs	20	L/s/ha		14	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	201.00	m ³	Total Storage	324.88 m ³
Total Rainwater Tank Credits		0.19	m ³		0.18 m ³
Storage Volume				Total	324.70 m ³
Storage Volume	Ext Detention Storage	200.81	m ³	Flood Detention Storage	123.90 m ³
OSD Discharges	Primary Outlet	16.12	L/s	Secondary Outlet	11.76 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	88	mm	Satisfactory	Satisfactory	73 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block C				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.8484	ha	8,484	m ²	
Total Roof Area	0.2851	ha	2,851	m ²	
Area of Site draining to OSD Storage	0.5099	ha	5,099	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.563	ha			
Area Bypassing Storage	0.3385	ha			
Area Bypassing / Residual Site Area	60.1%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	3				Satisfactory
Site Area per Dwelling	0.283	ha			
Roof Area per Dwelling	0.095	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	40%				
Adjusted SRDs	22	L/s/ha		30	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	207.86	m ³	Total Storage	335.97 m ³
Total Rainwater Tank Credits		0.18	m ³		0.16 m ³
Storage Volume				Total	335.80 m ³
Storage Volume	Ext Detention Storage	207.68	m ³	Flood Detention Storage	128.12 m ³
OSD Discharges	Primary Outlet	18.64	L/s	Secondary Outlet	25.30 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	95	mm	Satisfactory	Satisfactory	107 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements					
Site Address	Building Block D					
Job No:	FG486					
Designer:	SAB					
Telephone:	(02) 9929 4466					
Site Data						
OSD Area:	Upper Parramatta River Catchment					
L.G.A	Parramatta City Council					
Site Area	1.3294	ha	13,294	m ²		
Total Roof Area	0.4648	ha	4,648	m ²		
Area of Site draining to OSD Storage	0.8639	ha	8,639	m ²	Increase Area to Reduce Bypass	
Residual Site Area (Lot Area - Roof Area)	0.865	ha				
Area Bypassing Storage	0.4655	ha				
Area Bypassing / Residual Site Area	53.8%				Unacceptable - Exceeds 30% 30% Max	
No. of Dwellings on Site	5				Satisfactory	
Site Area per Dwelling	0.266	ha				
Roof Area per Dwelling	0.093	ha				
Basic OSD Parameters						
		Extended Detention			Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396	m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150	L/s/ha
OSD Tank Bypass						
Residual Lot Capture in OSD Tank	46%					
Adjusted SRDs	24	L/s/ha		42	L/s/ha	
OSD Calculations						
		Extended Detention			Detention	
Basic SSR Volume	Ext Detention Storage	325.70	m ³	Total Storage	526.44	m ³
Total Rainwater Tank Credits		0.30	m ³		0.27	m ³
Storage Volume				Total	526.17	m ³
Storage Volume	Ext Detention Storage	325.41	m ³	Flood Detention Storage	200.76	m ³
OSD Discharges	Primary Outlet	31.70	L/s	Secondary Outlet	56.26	L/s
RL of Top Water Level of Storage	10.100	m		10.300	m	
RL of Orifice Centre-line	9.100	m		9.000	m	
Number of Orifices	1			1		
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI	
Downstream FL - RL of Orifice Centre-line	-0.10	Satisfactory		Satisfactory		0.00 m
Design Head to Orifice Centre	1.000	m	TWL Ext Detn Storage - RL Orifice		1.100	m
Calculated Orifice Diameter	123	mm	Satisfactory		160	mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block E				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.9496	ha	9,496	m ²	
Total Roof Area	0.3431	ha	3,431	m ²	
Area of Site draining to OSD Storage	0.5741	ha	5,741	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.607	ha			
Area Bypassing Storage	0.3755	ha			
Area Bypassing / Residual Site Area	61.9%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	3				Satisfactory
Site Area per Dwelling	0.317	ha			
Roof Area per Dwelling	0.114	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	38%				
Adjusted SRDs	21	L/s/ha		26	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	232.65	m ³	Total Storage	376.04 m ³
Total Rainwater Tank Credits		0.12	m ³		0.11 m ³
Storage Volume				Total	375.93 m ³
Storage Volume	Ext Detention Storage	232.53	m ³	Flood Detention Storage	143.40 m ³
OSD Discharges	Primary Outlet	20.35	L/s	Secondary Outlet	24.86 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	99	mm	Satisfactory	Satisfactory	107 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block EA				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.5012	ha	15,012	m ²	
Total Roof Area	0.4376	ha	4,376	m ²	
Area of Site draining to OSD Storage	0.8992	ha	8,992	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	1.064	ha			
Area Bypassing Storage	0.602	ha			
Area Bypassing / Residual Site Area	56.6%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	5				Satisfactory
Site Area per Dwelling	0.300	ha			
Roof Area per Dwelling	0.088	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	43%				
Adjusted SRDs	23	L/s/ha		37	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	367.79	m ³	Total Storage	594.48 m ³
Total Rainwater Tank Credits		0.39	m ³		0.36 m ³
Storage Volume				Total	594.12 m ³
Storage Volume	Ext Detention Storage	367.40	m ³	Flood Detention Storage	226.71 m ³
OSD Discharges	Primary Outlet	34.56	L/s	Secondary Outlet	55.24 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	129	mm	Satisfactory	Satisfactory	159 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block EB				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.3975	ha	13,975	m ²	
Total Roof Area	0.5079	ha	5,079	m ²	
Area of Site draining to OSD Storage	0.9387	ha	9,387	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.890	ha			
Area Bypassing Storage	0.4588	ha			
Area Bypassing / Residual Site Area	51.6%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	6				Satisfactory
Site Area per Dwelling	0.233	ha			
Roof Area per Dwelling	0.085	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		48%			
Adjusted SRDs		25	L/s/ha	47	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	342.39	m ³	Total Storage	553.41 m ³
Total Rainwater Tank Credits		0.40	m ³		0.37 m ³
Storage Volume				Total	553.04 m ³
Storage Volume	Ext Detention Storage	341.99	m ³	Flood Detention Storage	211.05 m ³
OSD Discharges	Primary Outlet	34.28	L/s	Secondary Outlet	65.48 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory		0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		128	mm	Satisfactory	173 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements						
Site Address	Building Block EC						
Job No:	FG486						
Designer:	SAB						
Telephone:	(02) 9929 4466						
Site Data							
OSD Area:	Upper Parramatta River Catchment						
L.G.A	Parramatta City Council						
Site Area	0.4060533	ha	4,061	m ²			
Total Roof Area	0.11	ha	1,100	m ²			
Area of Site draining to OSD Storage	0.22647037	ha	2,265	m ²	Increase Area to Reduce Bypass		
Residual Site Area (Lot Area - Roof Area)	0.296	ha					
Area Bypassing Storage	0.17958292	ha					
Area Bypassing / Residual Site Area	60.7%				Unacceptable - Exceeds 30% Satisfactory	30% Max	
No. of Dwellings on Site	1						
Site Area per Dwelling	0.406	ha					
Roof Area per Dwelling	0.110	ha					
Basic OSD Parameters							
		Extended Detention		Detention			
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396	m ³ /ha	
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150	L/s/ha	
OSD Tank Bypass							
Residual Lot Capture in OSD Tank		39%					
Adjusted SRDs		22	L/s/ha		29	L/s/ha	
OSD Calculations							
		Extended Detention		Detention			
Basic SSR Volume	Ext Detention Storage	99.48	m ³	Total Storage	160.80	m ³	
Total Rainwater Tank Credits		0.06	m ³		0.05	m ³	
Storage Volume				Total	160.74	m ³	
Storage Volume	Ext Detention Storage	99.42	m ³	Flood Detention Storage	61.32	m ³	
OSD Discharges	Primary Outlet	8.85	L/s	Secondary Outlet	11.65	L/s	
RL of Top Water Level of Storage		10.100	m		10.300	m	
RL of Orifice Centre-line		9.100	m		9.000	m	
Number of Orifices		1			1		
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00	100 yr ARI	
Downstream FL - RL of Orifice Centre-line		-0.10		Satisfactory	Satisfactory	0.00	m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100	m	
Calculated Orifice Diameter		65	mm	Satisfactory	Satisfactory	73	mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements						
Site Address	Building Block F						
Job No:	FG486						
Designer:	SAB						
Telephone:	(02) 9929 4466						
Site Data							
OSD Area:	Upper Parramatta River Catchment						
L.G.A	Parramatta City Council						
Site Area	1.5071	ha	15,071	m ²			
Total Roof Area	0.5552	ha	5,552	m ²			
Area of Site draining to OSD Storage	1.0097	ha	10,097	m ²	Increase Area to Reduce Bypass		
Residual Site Area (Lot Area - Roof Area)	0.952	ha					
Area Bypassing Storage	0.4974	ha					
Area Bypassing / Residual Site Area	52.3%					Unacceptable - Exceeds 30%	30% Max
No. of Dwellings on Site	6						
Site Area per Dwelling	0.251	ha					
Roof Area per Dwelling	0.093	ha					
Basic OSD Parameters							
		Extended Detention			Detention		
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396	m ³ /ha	
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150	L/s/ha	
OSD Tank Bypass							
Residual Lot Capture in OSD Tank	48%						
Adjusted SRDs	24	L/s/ha	45	L/s/ha			
OSD Calculations							
		Extended Detention			Detention		
Basic SSR Volume	Ext Detention Storage	369.24	m ³	Total Storage	596.81	m ³	
Total Rainwater Tank Credits	0.34		m ³	0.31		m ³	
Storage Volume	368.90		m ³	Total	596.50	m ³	
Storage Volume	Ext Detention Storage	368.90	m ³	Flood Detention Storage	227.60	m ³	
OSD Discharges	Primary Outlet	36.66	L/s	Secondary Outlet	68.56	L/s	
RL of Top Water Level of Storage	10.100	m	10.300	m			
RL of Orifice Centre-line	9.100	m	9.000	m			
Number of Orifices	1						
Estimated Downstream Flood Level	9.00	1.5 yr ARI	9.00	100 yr ARI			
Downstream FL - RL of Orifice Centre-line	-0.10	Satisfactory		Satisfactory		0.00 m	
Design Head to Orifice Centre	1.000	m	TWL Ext Detn Storage - RL Orifice		1.100	m	
Calculated Orifice Diameter	133	mm	Satisfactory	Satisfactory	177	mm	

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block G				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.24651879 ha	12,465	m ²		
Total Roof Area	0.431 ha	4,310	m ²		
Area of Site draining to OSD Storage	0.88674177 ha	8,867	m ²	Increase Area to Reduce Bypass	
Residual Site Area (Lot Area - Roof Area)	0.816 ha				
Area Bypassing Storage	0.35977701 ha				
Area Bypassing / Residual Site Area	44.1%			Unacceptable - Exceeds 30% 30% Max	
No. of Dwellings on Site	6			Satisfactory	
Site Area per Dwelling	0.208 ha				
Roof Area per Dwelling	0.072 ha				
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		56%			
Adjusted SRDs		27	L/s/ha	62	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	305.40	m ³	Total Storage	493.62 m ³
Total Rainwater Tank Credits		0.54	m ³		0.50 m ³
Storage Volume				Total	493.12 m ³
Storage Volume	Ext Detention Storage	304.86	m ³	Flood Detention Storage	188.26 m ³
OSD Discharges	Primary Outlet	33.36	L/s	Secondary Outlet	76.99 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		126	mm	Satisfactory	Satisfactory 188 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block H				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.745	ha	7,450	m ²	
Total Roof Area	0.26204701	ha	2,620	m ²	
Area of Site draining to OSD Storage	0.427	ha	4,270	m ²	
Residual Site Area (Lot Area - Roof Area)	0.483	ha			
Area Bypassing Storage	0.318	ha			
Area Bypassing / Residual Site Area	65.8%				
No. of Dwellings on Site	3				
Site Area per Dwelling	0.248	ha			
Roof Area per Dwelling	0.087	ha			
				Increase Area to Reduce Bypass	
				Unacceptable - Exceeds 30% 30% Max	
				Satisfactory	
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		34%			
Adjusted SRDs		20	L/s/ha	18	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	182.53	m ³	Total Storage	295.02 m ³
Total Rainwater Tank Credits		0.19	m ³		0.18 m ³
Storage Volume				Total	294.84 m ³
Storage Volume	Ext Detention Storage	182.33	m ³	Flood Detention Storage	112.51 m ³
OSD Discharges	Primary Outlet	15.08	L/s	Secondary Outlet	13.64 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		85	mm	Satisfactory	Satisfactory 79 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block K				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	0.7427	ha	7,427	m ²	
Total Roof Area	0.2406	ha	2,406	m ²	
Area of Site draining to OSD Storage	0.4237	ha	4,237	m ²	Increase Area to Reduce Bypass
Residual Site Area (Lot Area - Roof Area)	0.502	ha			
Area Bypassing Storage	0.319	ha			
Area Bypassing / Residual Site Area	63.5%				Unacceptable - Exceeds 30% 30% Max
No. of Dwellings on Site	3				Satisfactory
Site Area per Dwelling	0.248	ha			
Roof Area per Dwelling	0.080	ha			
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank	36%				
Adjusted SRDs	21	L/s/ha		23	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	181.96	m ³	Total Storage	294.11 m ³
Total Rainwater Tank Credits		0.24	m ³		0.22 m ³
Storage Volume				Total	293.88 m ³
Storage Volume	Ext Detention Storage	181.72	m ³	Flood Detention Storage	112.16 m ³
OSD Discharges	Primary Outlet	15.55	L/s	Secondary Outlet	17.03 L/s
RL of Top Water Level of Storage	10.100	m		10.300	m
RL of Orifice Centre-line	9.100	m		9.000	m
Number of Orifices	1			1	
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI
Downstream FL - RL of Orifice Centre-line	-0.10		Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter	86	mm	Satisfactory	Satisfactory	88 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block N				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	2.3152	ha	23,152	m ²	
Total Roof Area	0.7446	ha	7,446	m ²	
Area of Site draining to OSD Storage	1.8068	ha	18,068	m ²	
Residual Site Area (Lot Area - Roof Area)	1.571	ha			
Area Bypassing Storage	0.5084	ha			
Area Bypassing / Residual Site Area	32.4%				
No. of Dwellings on Site	7				
Site Area per Dwelling	0.331	ha			
Roof Area per Dwelling	0.106	ha			
				Increase Area to Reduce Bypass	
				Unacceptable - Exceeds 30% Satisfactory	
				30% Max	
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		68%			
Adjusted SRDs		30	L/s/ha	85	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	567.22	m ³	Total Storage	916.82 m ³
Total Rainwater Tank Credits		0.37	m ³		0.33 m ³
Storage Volume				Total	916.49 m ³
Storage Volume	Ext Detention Storage	566.86	m ³	Flood Detention Storage	349.63 m ³
OSD Discharges	Primary Outlet	70.13	L/s	Secondary Outlet	197.39 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10		Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		183	mm	Satisfactory	300 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block N1				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.08080953 ha	10,808	m ²		
Total Roof Area	0.35472725 ha	3,547	m ²		
Area of Site draining to OSD Storage	0.782411 ha	7,824	m ²	Increase Area to Reduce Bypass	
Residual Site Area (Lot Area - Roof Area)	0.726 ha				
Area Bypassing Storage	0.29839854 ha				
Area Bypassing / Residual Site Area	41.1%			Unacceptable - Exceeds 30% 30% Max	
No. of Dwellings on Site	7			Satisfactory	
Site Area per Dwelling	0.154 ha				
Roof Area per Dwelling	0.051 ha				
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		59%			
Adjusted SRDs		28	L/s/ha	68	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	264.80	m ³	Total Storage	428.00 m ³
Total Rainwater Tank Credits		1.13	m ³		1.10 m ³
Storage Volume				Total	426.90 m ³
Storage Volume	Ext Detention Storage	263.67	m ³	Flood Detention Storage	163.23 m ³
OSD Discharges	Primary Outlet	29.91	L/s	Secondary Outlet	73.29 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		120	mm	Satisfactory	Satisfactory 183 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	Building Block N2				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.24568959 ha	12,457	m ²		
Total Roof Area	0.38980452 ha	3,898	m ²		
Area of Site draining to OSD Storage	1.02436938 ha	10,244	m ²	Satisfactory	
Residual Site Area (Lot Area - Roof Area)	0.856 ha				
Area Bypassing Storage	0.2213202 ha				
Area Bypassing / Residual Site Area	25.9%			Satisfactory 30% Max	
No. of Dwellings on Site	7			Satisfactory	
Site Area per Dwelling	0.178 ha				
Roof Area per Dwelling	0.056 ha				
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		74%			
Adjusted SRDs		32	L/s/ha	98	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	305.19	m ³	Total Storage	493.29 m ³
Total Rainwater Tank Credits		1.03	m ³		0.99 m ³
Storage Volume				Total	492.30 m ³
Storage Volume	Ext Detention Storage	304.17	m ³	Flood Detention Storage	188.14 m ³
OSD Discharges	Primary Outlet	40.16	L/s	Secondary Outlet	122.43 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		139	mm	Satisfactory	Satisfactory
					236 mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements					
Site Address	Building Block O					
Job No:	FG486					
Designer:	SAB					
Telephone:	(02) 9929 4466					
Site Data						
OSD Area:	Upper Parramatta River Catchment					
L.G.A	Parramatta City Council					
Site Area	1.4532	ha	14,532	m ²		
Total Roof Area	0.5657	ha	5,657	m ²		
Area of Site draining to OSD Storage	1.0393	ha	10,393	m ²	Increase Area to Reduce Bypass	
Residual Site Area (Lot Area - Roof Area)	0.888	ha				
Area Bypassing Storage	0.4139	ha				
Area Bypassing / Residual Site Area	46.6%				Unacceptable - Exceeds 30% 30% Max	
No. of Dwellings on Site	6				Satisfactory	
Site Area per Dwelling	0.242	ha				
Roof Area per Dwelling	0.094	ha				
Basic OSD Parameters						
		Extended Detention			Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396	m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150	L/s/ha
OSD Tank Bypass						
Residual Lot Capture in OSD Tank	53%					
Adjusted SRDs	26	L/s/ha		57	L/s/ha	
OSD Calculations						
		Extended Detention			Detention	
Basic SSR Volume	Ext Detention Storage	356.03	m ³	Total Storage	575.47	m ³
Total Rainwater Tank Credits		0.31	m ³		0.29	m ³
Storage Volume				Total	575.18	m ³
Storage Volume	Ext Detention Storage	355.72	m ³	Flood Detention Storage	219.46	m ³
OSD Discharges	Primary Outlet	37.80	L/s	Secondary Outlet	82.44	L/s
RL of Top Water Level of Storage	10.100	m		10.300	m	
RL of Orifice Centre-line	9.100	m		9.000	m	
Number of Orifices	1			1		
Estimated Downstream Flood Level	9.00	1.5 yr ARI		9.00	100 yr ARI	
Downstream FL - RL of Orifice Centre-line	-0.10	Satisfactory		Satisfactory		0.00 m
Design Head to Orifice Centre	1.000	m		TWL Ext Detn Storage - RL Orifice		1.100 m
Calculated Orifice Diameter	135	mm	Satisfactory		Satisfactory	
					194	mm

On-Site Detention Calculation Sheet for Upper Parramatta River Catchment HED Secondary Outlet

Project:	Melrose Park PP Site OSD Requirements				
Site Address	School				
Job No:	FG486				
Designer:	SAB				
Telephone:	(02) 9929 4466				
Site Data					
OSD Area:	Upper Parramatta River Catchment				
L.G.A	Parramatta City Council				
Site Area	1.45991356 ha	14,599	m ²		
Total Roof Area	0.23982888 ha	2,398	m ²		
Area of Site draining to OSD Storage	0.99161949 ha	9,916	m ²	Increase Area to Reduce Bypass	
Residual Site Area (Lot Area - Roof Area)	1.220 ha				
Area Bypassing Storage	0.46829407 ha				
Area Bypassing / Residual Site Area	38.4%			Unacceptable - Exceeds 30% 30% Max	
No. of Dwellings on Site	1			Satisfactory	
Site Area per Dwelling	1.460 ha				
Roof Area per Dwelling	0.240 ha				
Basic OSD Parameters					
		Extended Detention		Detention	
Basic SSR Vols	Ext Detention Storage	245	m ³ /ha	Total Storage	396 m ³ /ha
Basic SRDs	Primary Outlet	40	L/s/ha	Secondary Outlet	150 L/s/ha
OSD Tank Bypass					
Residual Lot Capture in OSD Tank		62%			
Adjusted SRDs		28	L/s/ha	73	L/s/ha
OSD Calculations					
		Extended Detention		Detention	
Basic SSR Volume	Ext Detention Storage	357.68	m ³	Total Storage	578.13 m ³
Total Rainwater Tank Credits		0.03	m ³		0.02 m ³
Storage Volume				Total	578.10 m ³
Storage Volume	Ext Detention Storage	357.65	m ³	Flood Detention Storage	220.45 m ³
OSD Discharges	Primary Outlet	41.59	L/s	Secondary Outlet	106.92 L/s
RL of Top Water Level of Storage		10.100	m		10.300 m
RL of Orifice Centre-line		9.100	m		9.000 m
Number of Orifices		1			1
Estimated Downstream Flood Level		9.00	1.5 yr ARI		9.00 100 yr ARI
Downstream FL - RL of Orifice Centre-line		-0.10	Satisfactory	Satisfactory	0.00 m
Design Head to Orifice Centre		1.000	m	TWL Ext Detn Storage - RL Orifice	1.100 m
Calculated Orifice Diameter		141	mm	Satisfactory	Satisfactory 221 mm